November 15, 2024

Addendum 1

Bid No. 25-005-2025-PWI-014

Metal Building – 50' x 120' Pre-engineered Metal Building with Foundation

Electric Storage Building Addition for the

City of Fairhope – Electric Utility

Public Works Project No. 2025-PWI 014

Addendum 1 contains questions and answers submitted via email and during the non-mandatory pre-bid meeting held on Thursday, November 14, 2024 at 10:00 a.m.

This bid will be opened at the City of Fairhope's City Services and Public Utilities Building, 555 South Section Street, Fairhope, AL 36532 at 10:30 AM on Thursday, November 21, 2024. Vendors shall acknowledge this Addendum 1 on their submitted Bid Response Form.

Questions Submitted Via Email:

1. I couldn't find the digital plans for the new building or the existing structure it will be attached to on your website. Could you please provide those plans?
   A. Conceptual plans are provided in the bid specifications. The winning bidder will provide construction plans as part of the contract. The permit set for the existing structure that was built circa 2023 is attached.

2. Could you tell me who the general contractor was who built the original building you are adding on to?
   A. Mad Dash Inc. d/b/a Southern Steel Structures.

3. Is there a preferred bay spacing the architects want? The drawings show 30'. 20' might be more cost effective.
   A. Bay spacing shall be at the discretion of the design engineer – based on required structural design to meet code compliance. City has no preference.

4. 3" of insulation, correct?
   A. Correct.

5. Galvalume roof and painted walls and trim, correct?
   A. Correct – match existing.

Questions Submitted During the Non-Mandatory Pre-Bid Meeting:

1. What is defined as Heavy-Duty Roll Up doors - rolling steel doors (slatted) or commercial sheet doors – with operator?
    A. Budget does not allow for the more expensive slatted door – heavy duty commercial sheet door is the intent of the specification. Doors provided will have cast iron reduced drive hand operated chain hoist. The doors shall be Wind Load Rated per the engineered design and meet or exceed the specifications of the continuous sheet rolling door Model 2500 as manufactured by Janus International – specification attached.

2. Is there a domestic content requirement in this bid?
    A. No

3. What is the project budget range?
    A. $150,000.00 to $180,000.00

4. Who was the GeoTech used from the original project?
    A. The City self-performed the design and construction of the foundation/slab system of the original project. Sub-base conditions were determined to be satisfactory based on visual inspection of compaction. No GeoTech testing was warranted or used. Note: Winning Bid will include all engineering, site surveying, geotechnical investigation.

5. Are building permits required and will there be permit fees?
    A. The winning bidder will be required to file and apply for all required Building Permits through the City's Citizen Serve Portal/System – No permit fees will be charged since this is a City Project.

6. Will the opening (access to the new addition) at the end of the building require additional structural framing?
    A. It is the intent of the City for the east side of the north wall to be open to the new addition. It is anticipated that no additional structural framing should be required with the removal of the partial wall panel skins and girts. However, it will be the responsibility of the winning bidder's design engineer to make that determination based on all specified Code requirements.

Builder/Contractor Responsibilities

Drawing Validity – These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

Builder Acceptance of Drawings – Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (AISC COSP April 2010 Section 4.4.1)

Code Official Approval – It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Building Erection – The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector (AISC COSP April 2010 Section 7.10.3).

Discrepancies – Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC COSP April 2010 Section 3.3)

Materials by Others – All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturers assumptions will govern.

Modification of the Metal Building from Plans – The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

Foundation Design
The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 06 Sections 3.2.2 and A3)

Shimming – "In accordance with Section 6.10 of Chapter 4 Common Industry Practices in the Metal Building Systems Manual, shimming is a normal part of erection and is not subject to claim."

CECO Building Systems
For questions or assistance Concerning Erection call or Email:
1-844-840-4603
Monday - Friday 7:30am to 5:00pm
FIELD.SERVICES@CORNERSTONE-BB.COM

Quality Metal Building Systems From Your Construction Professionals

MBMA MEMBER · IAS ACCREDITED · FM APPROVED PBR, DOUBLE-LOK, SUPERLOK, LS-36, CFR, GFIWEIL-PLANK, V-RIB, 7.2 INSUL-RIB, MESA, FLUTE, STRATEX, SANTA FE, ARCH, FLAT · MT. PLEASANT IOWA · MIAMI DADE MAP, PBR, BATTENLOK HS, DOUBLE-LOK, SUPERLOK · ROOF SYSTEMS MAP, PBR, PBU, LOKSEAM, DOUBLE-LOK, ULTRA-DEK, BATTENLOK HS, SUPERLOK, CFR, 7.2 PANEL

ENGINEERING DESIGN CRITERIA

Building Code ....: 2018 IBC
Building Risk Category ....: Normal (Risk Category II)
Roof Dead Load
  Superimposed ....: 2.48 psf
  Collateral ....: 0.50 psf
  (0.50 psf Other)
Roof Live Load ....: 20.00 psf reduction allowed

Wind
  Ultimate Wind Speed (Vult) .: 160.00 mph
  Nominal Wind Speed (Vasd) ..: 123 mph (IBC section 1609.3.1)
  Serviceability Wind Speed ..: 83 mph
  Ground Elevation Factor ....: 1.00 (110 ft ASL)
  Wind Exposure Category .....: B
  Exposure Coefficient (MWFRS): 0.701
  Enclosure Classification ...: Partially Enclosed Building
  Internal Pressure Coef (GCpi): 0.55/-0.55
  Wall Loads for components not provided by building manufacturer
  Zone 5 Areas (within 5.00' of corner) : 48.71 psf pressure -60.80 psf suction
  Zone 4 Areas (away from corners) : 48.71 psf pressure -51.73 psf suction
  These values are the maximum values required based on a 10 sq ft area.
  Components with larger areas may have lower wind loads.

Seismic
  Seismic Importance Factor (Ie): 1.00
  Seismic Design Category ....: B
  Soil Site Class ....: D Stiff Soil (Default)
  Ss ....: 0.089 g   Sds ....: 0.095 g
  S1 ....: 0.057 g   Sd1 ....: 0.092 g
  Analysis Procedure ....: Equivalent Lateral Force
  Column Line   All
  Basic Force Resisting System   H
  Response Modification Coefficient (R)   3.00
  Seismic Response Coefficient (Cs)   0.03
  Design Base Shear in kips (V)   0.88
  Basic Structural System (from ASCE 7-16 Table 12.2-1)
   H - Steel System not Specifically Detailed for Seismic Resistance

DEFLECTION CRITERIA

The material supplied by the manufacturer has been designed with the following minimum deflection criteria. The actual deflection may be less depending on actual load and actual member length.

BUILDING DEFLECTION LIMITS ......: BLDG-A

Roof Limits		Rafters	Purlins	Panels
Live:	L/	180	150	60
Serviceability Wind:	L/	180	180	60
Total Gravity:	L/	120	120	60
Total Uplift:	L/	N/A	N/A	60

Frame Limits		Sidesway	Portal Frame Sidesway
Live:	H/	60	
Serviceability Wind:	H/	60	
Seismic Drift:	H/	40	40
Portal Serviceability Wind:	H/	N/A	60
Total Gravity:	H/	60	
Service Seismic:	H/	40	40

Wall Limits		Limit
Total Wind Panels:	L/	60
Total Wind Girts:	L/	90
Total Wind EW Columns:	L/	120

The Service Seismic limit as shown here is at service level loads.

PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, or ASTM A1011 with 55 ksi min. yield, except flanges wider than 12" and thicker than 3/8", all flanges thicker than 1", and all webs thicker than 3/8" are 50 ksi min. yield. Rod X-bracing conforms to ASTM A529 or ASTM A572 with 50 ksi min. yield. Cable X-bracing conforms to ASTM A475 7 Strand Extra High-Strength grade. Hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with 50 ksi min. yield. Hot rolled angles, other than flange braces, conform to ASTM A36 minimum. Round and rectangular HSS conforms to ASTM A500 Grade B. Cold-formed steel secondary framing Members conform to ASTM A1011 or ASTM A653 Grade 55 with 55 ksi min. yield. For Canada, material properties conform to CAN/CSA G40.20/G40.21 or equivalent.

Unless otherwise noted, special inspection of fabricated items is not required. Per IBC section 1704.2.5.1, fabricator is approved to perform such work without special inspection through maintenance of IAS AC 472 certification MB-136.

All bolted joints with A325 Type 1 bolts are specified as snug-tightened joints in accordance with the most recent edition of the RCSC Specification for Structural Joints Using ASTM A325 or A490 Bolts. Pre-tensioning methods, including turn-of-nut, calibrated wrench, twist-off-type tension-control bolts or direct-tension-indicator are NOT required. Installation inspection requirements for Snug Tight Bolts (Specification for Structural Joints Section 9.1) is suggested.

Design criteria as noted is as given within order documents and is applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the metal building manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for local provisions that may apply or for site specific parameters. The design criteria is supplied by the builder, project owner, or an Architect and/or Engineer of Record for the overall construction project.

Framed openings, walk doors, and open areas shall be located in the bay and elevation as shown in the erection drawings. The cutting or removal of girts shown on the erection drawings due to the addition of framed openings, walk doors, or open areas not shown may void the design certifications supplied by the metal building manufacturer.

This jobsite is located in a hurricane prone region with wind speeds of 160 mph or greater. In order to maintain the partial enclosed classification and design for wind all doors, windows and wall mounted light transmitting panels (LTP) provided by the metal building manufacturer shall be protected by impact resistant coverings. The material may include but is not limited to 7/16 structural wood panels as prescribed by the local building code. The customer's Design Professional, not metal building manufacturer engineer, is responsible for determining the adequacy of material acting as the impact resistant covering by others and attachment to the material provided by the metal building manufacturer. This structure has not been designed to withstand the additional internal pressure required by Code as a partially enclosed condition in the absence of impact resistant coverings.

Drawing Index

Revision	Date	Description	By	Ck'd

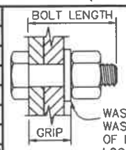
Download panel installation manuals from: www.CBBmanuals.com

Descargue los manuales de instalación del panel desde: www.CBBmanuals.com

BUILDING DESCRIPTIONS				
Building ID	Width	Length	Height	Slope
Building A	50'–0	100'–0	18'–0	1:12

$\frac{1}{2}$"ø A325 BOLT GRIP TABLE (UNLESS NOTED)

GRIP	LENGTH
0 TO 9/16"	1 1/4" F.T.
Over 9/16" TO 1 1/16"	1 3/4" F.T.
Over 1 1/16" TO 1 5/16"	2"
Over 1 5/16" TO 1 9/16"	2 1/4"
Over 1 9/16" TO 1 13/16"	2 1/2"
Over 1 13/16" TO 2 1/16"	2 3/4"
LOCATIONS OF BOLTS LONGER THAN 2 3/4" NOTED ON ERECTION DRAWINGS	
F.T. DENOTES FULLY THREADED	



NOTE: FULL THREAD ENGAGEMENT IS DEEMED TO HAVE BEEN MET WHEN THE END OF THE BOLT IS FLUSH WITH THE FACE OF THE NUT.

WASHER REQUIRED ONLY WHEN SPECIFIED. WASHER MAY BE LOCATED UNDER HEAD OF BOLT, UNDER NUT, OR AT BOTH AT LOCATIONS NOTED ON ERECTION DRAWINGS. ADD 5/32" FOR EACH WASHER TO MATERIAL THICKNESS TO DETERMINE GRIP.

CECO Building Systems
Columbus, MS. (662) 243-6400
Mount Pleasant, IA. (319) 385-8001
Rocky Mount, NC. (252) 977-2131
www.cecobuildings.com

Customer:
MAD DASH INC. DBA SOUTHERN STEEL ST
2210 MAIN ST STE G
DAPHNE, AL 36526-4546 US
ATTN: CORNELIUS LINDA

Project Name & Location:
MAD DASH INC. DBA SOUTHERN STEEL ST
19121 YOUNG ST
FAIRHOPE, AL 36532-1619 US

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale: NOT TO SCALE
Drawn by: ALN 2/3/23
Checked by: ABE 2/7/23
Project Engineer: JDM
Job Number: 19-B-27589-1
Sheet Number: **E1 of 10**

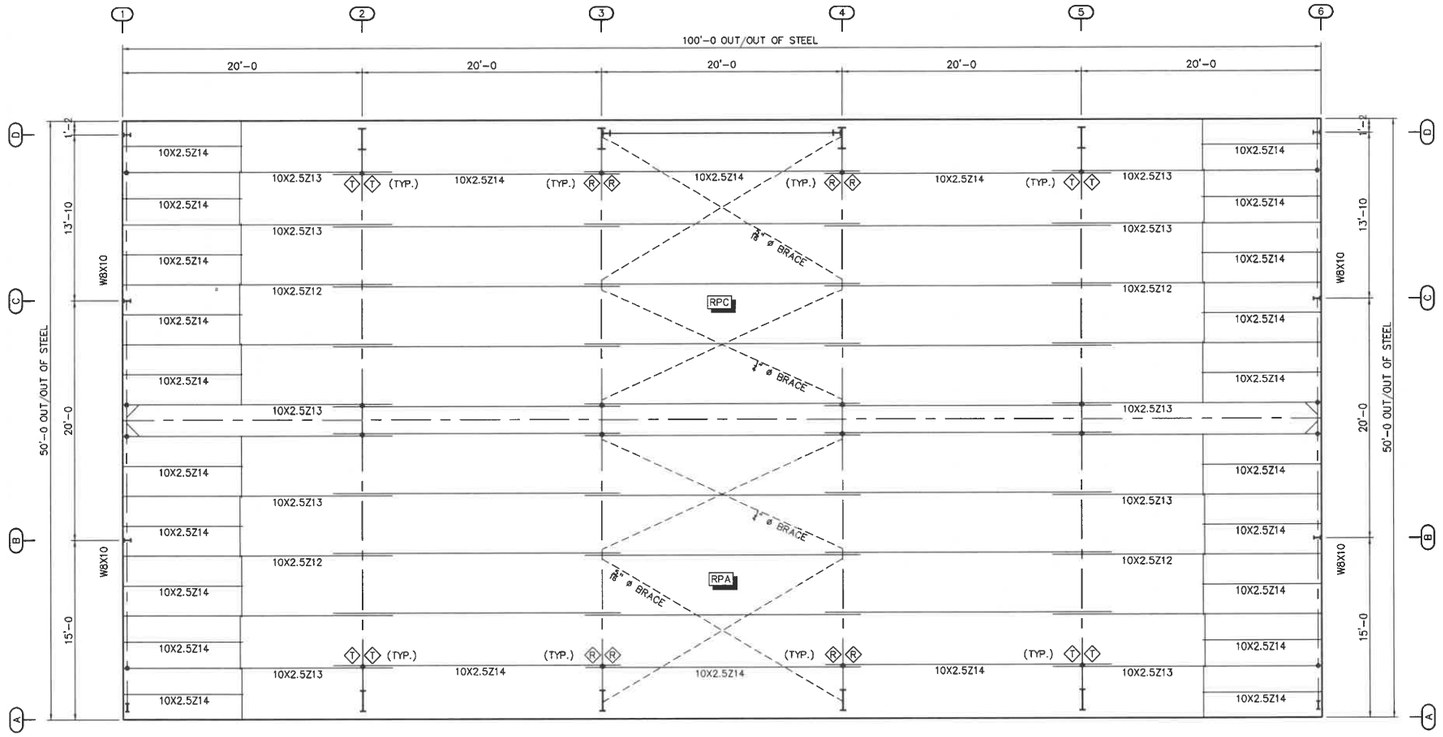
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

M.W. Custer, P.E.
Alabama P.E. PE21880

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:02:09 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

● – DENOTES: CLIP LOCATION
SC90 AT 8" PURLINS
SC92 AT 10" PURLINS
SC94 AT 12" PURLINS



ROOF FRAMING PLAN

SWC
EWB KEY PLAN EWD
SWA

ZEE SECTION LAP TABLE			
SYMBOL	LAP LENGTH	SYMBOL	LAP LENGTH
Ⓠ	-0'-0½"	Ⓣ	2'-5½"
Ⓢ	0'-3½"	Ⓥ	3'-1½"
Ⓡ	1'-5½"	REFER TO CF01122	

Revision	Date	Description	By	Ck'd

CECO Building Systems
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Project Name & Location:
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STE G
5121 YOUNG ST
FAIRHOPE, AL 36532-1619 US

Customer:
MAD DASH INC. DBA SOUTHERN
STE G
2210 MAIN ST
DAPHNE, AL 36526-4946 US
ATTN: CHELSEA

Drawing Status:
☐ Preliminary, Not For Construction
☐ For Approval, Not For Construction
☒ For Construction Permit
☐ For Erector Installation

Scale: NOT TO SCALE
Drawn by: ALN 2/3/23
Checked by: ABE 2/7/23
Project Engineer: JDM
Job Number: 19-B-27589-1
Sheet Number: E2 of 10

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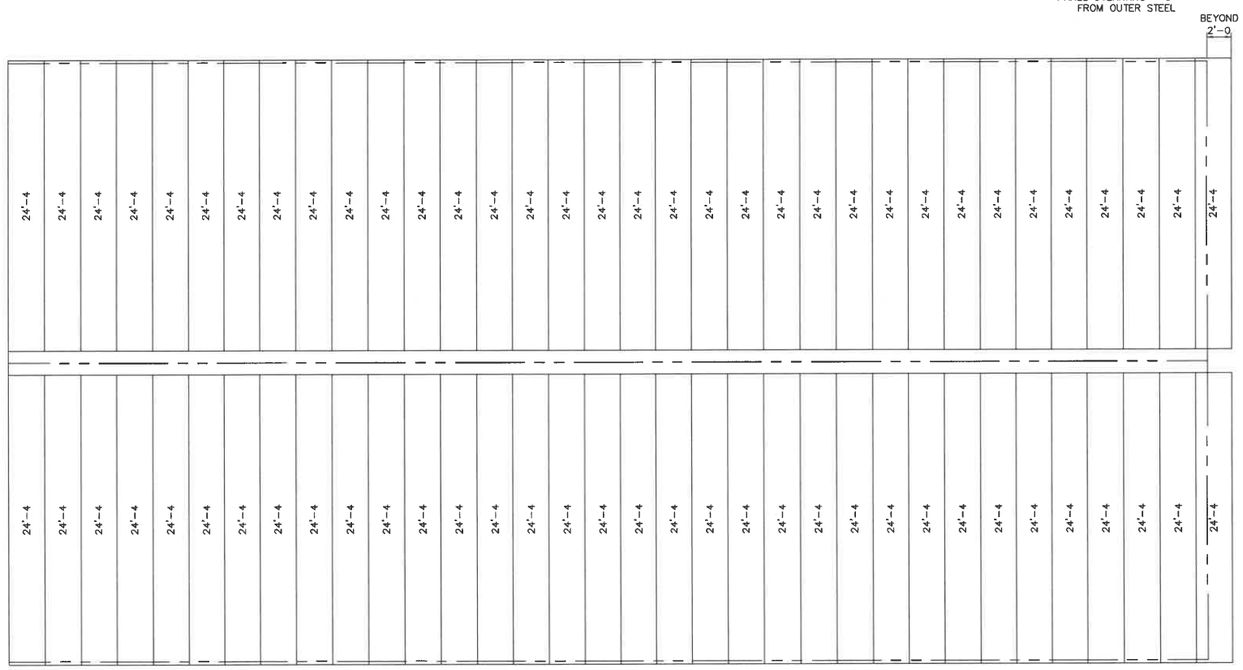
Alabama P.E. PE21880

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:02:21 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

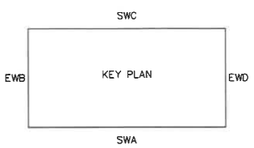
PBR ROOF PANELS ARE TO BE FIELD CUT IF THE PANELS EXTEND OUTSIDE OF THE ROOF PLANE, PANELS ARE NOT TO BE BACK LAPPED.

ROOF SHEETING PLANE 2
PANEL TYPE = PBR (GALVALUME)
PANEL OVERHANG = 3"
FROM OUTER STEEL



ROOF SHEETING PLAN

ROOF SHEETING PLANE 1
PANEL TYPE = PBR (GALVALUME)
PANEL OVERHANG = 3"
FROM OUTER STEEL



MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:02:32 -06'00'

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Mount Pleasant, IA (319) 385-8001
Rocky Mount, NC (252) 977-2131
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Project Name & Location:
MAD DASH INC. DBA SOUTHERN STEEL
1101 YOUNG ST
FAIRHOPE, AL 36532-1619 US

Customer:
MAD DASH INC. DBA SOUTHERN STEEL
2210 MAIN ST STE G
DAPHNE, AL 36526-4546 US
ATTN: CORNELIUS LINDA

Drawing Status:
☐ Preliminary
☐ Not For Construction
☒ For Construction Permit
☐ For Approval
☐ Not For Construction
☐ For Erector Installation

Scale:	NOT TO SCALE
Drawn by:	ALN 2/3/23
Checked by:	ABE 2/7/23
Project Engineer:	JDM
Job Number:	19-B-27589-1
Sheet Number:	E3 of 10

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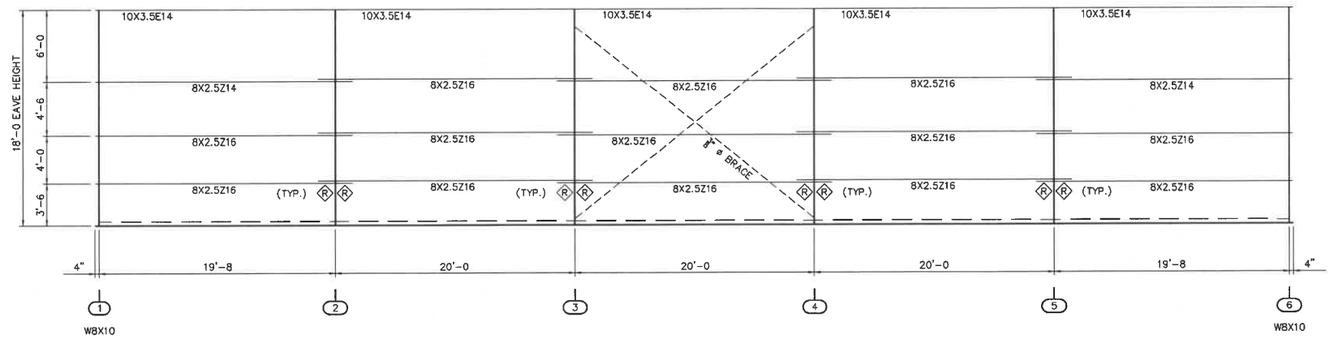
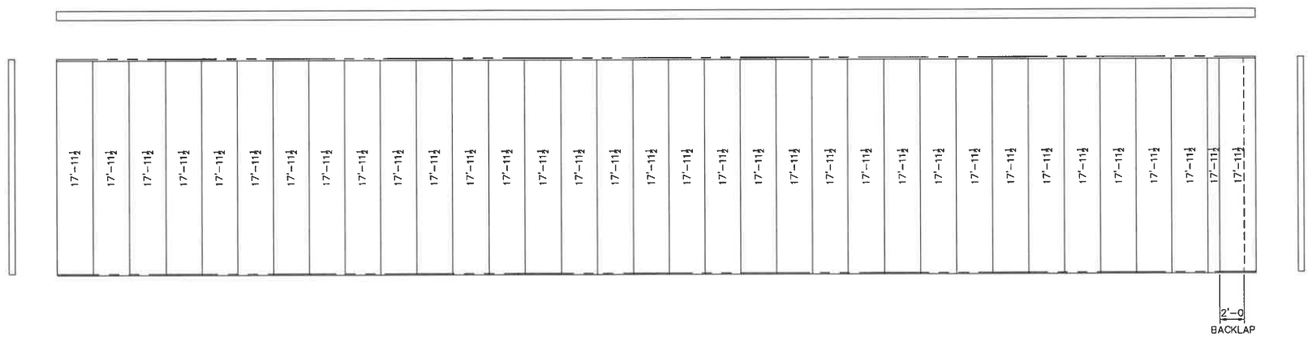
M.W. Custer, P.E.
Alabama P.E. PE21880

USE STANDARD WALL PROCEDURES TO ERECT THE SIDEWALL AND ENDWALL PANELS.

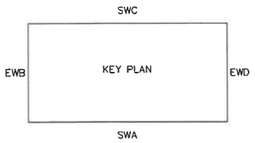


SIDEWALL ELEVATION "SWA" AT GRID LINE "A"



PBR WALL PANELS
PANEL COVERAGE = 3'-0
COLOR = ASH GRAY
PANEL PKG. REQ'D. = PBS-2
Field Cut Panel and Trim as required per Construction Details

WALL SHEETING ELEVATION "SWA"
BLDG "A"



ZEE SECTION LAP TABLE			
SYMBOL	LAP LENGTH	SYMBOL	LAP LENGTH
S	-0'-0¼"	T	2'-5½"
S	0'-3½"	V	3'-1¼"
R	1'-5½"	REFER TO CF01122	

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:02:42 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

Revision	Date	Description	By	Ck'd

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Project Name & Location:
MAD DASH INC. DBA SOUTHERN STEEL STRUCTURES
1917 YOUNG ST.
FAIRHOPE, AL 36532-1619 US

Customer:
MAD DASH INC. DBA SOUTHERN STEEL STRUCTURES
2210 MAIN ST STE G
DAPHNE, AL 36526-4546 US
ATTN: CHRIS HALL

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale:	NOT TO SCALE
Drawn by:	ALN 2/3/23
Checked by:	ABE 2/7/23
Project Engineer:	JDM
Job Number:	19-B-27589-1
Sheet Number:	E4 of 10

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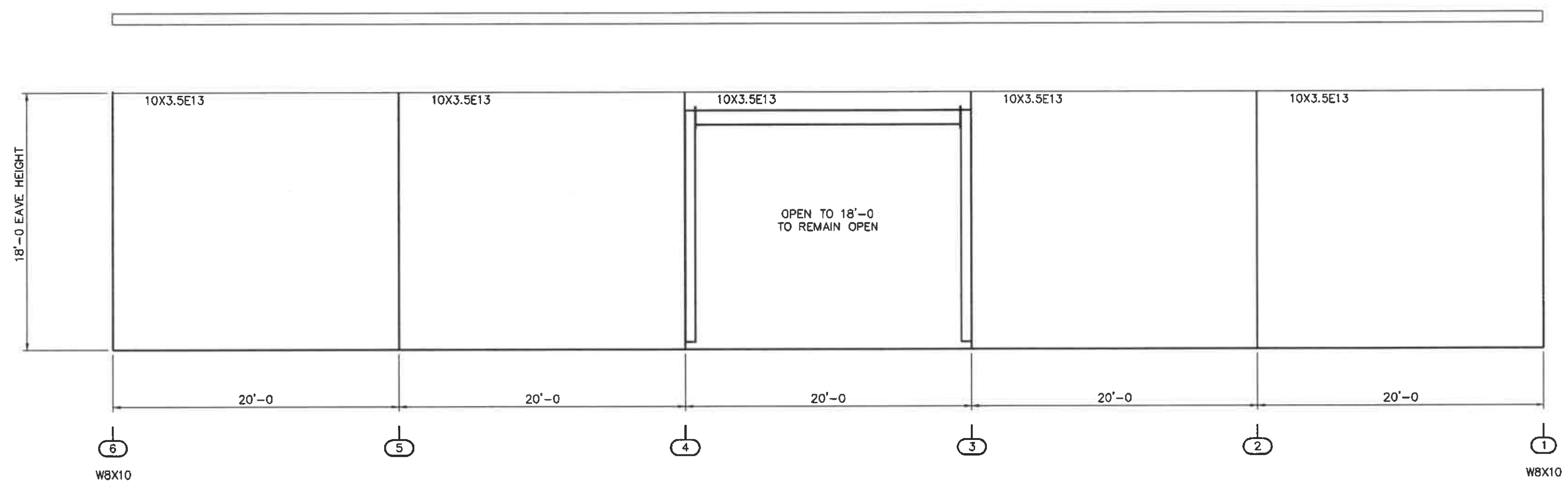
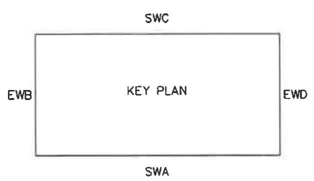
Alabama P.E. PE21880



SIDEWALL ELEVATION "SWC" AT GRID LINE "D"



ZEE SECTION LAP TABLE			
SYMBOL	LAP LENGTH	SYMBOL	LAP LENGTH
G	-0'-0¼"	T	2'-5¾"
S	0'-3¾"	V	3'-1¾"
R	1'-5¾"	REFER TO CF01122	

Revision	Date	Description	By	Ck'd

CECO Building Systems

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DAPHNE, AL 36526-4546 US
ATTN: CORNELIUS LINDA

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☐ For Erector Installation

Scale: NOT TO SCALE
Drawn by: ALN 2/3/23
Checked by: ABE 2/7/23
Project Engineer: JDM
Job Number: 19-B-27589-1
Sheet Number: E5 of 10

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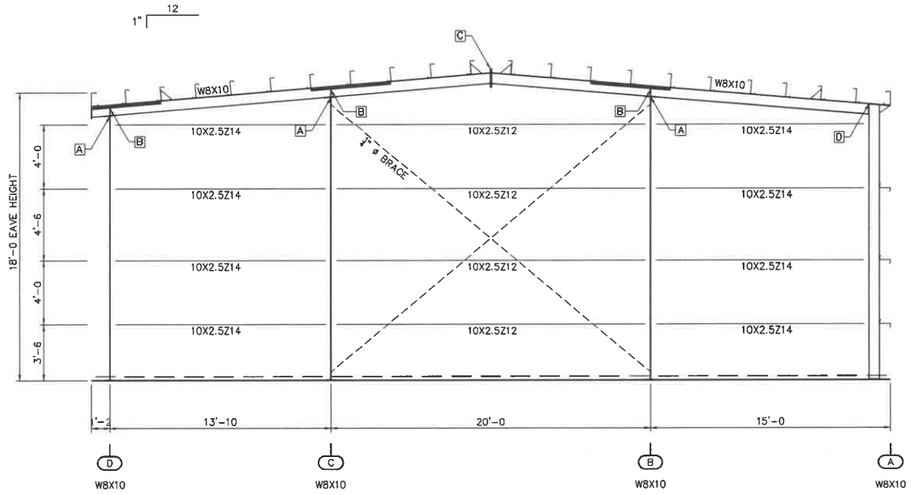
M.W. Custer, P.E.
Alabama P.E. PE21880

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:02:52 -06'00'

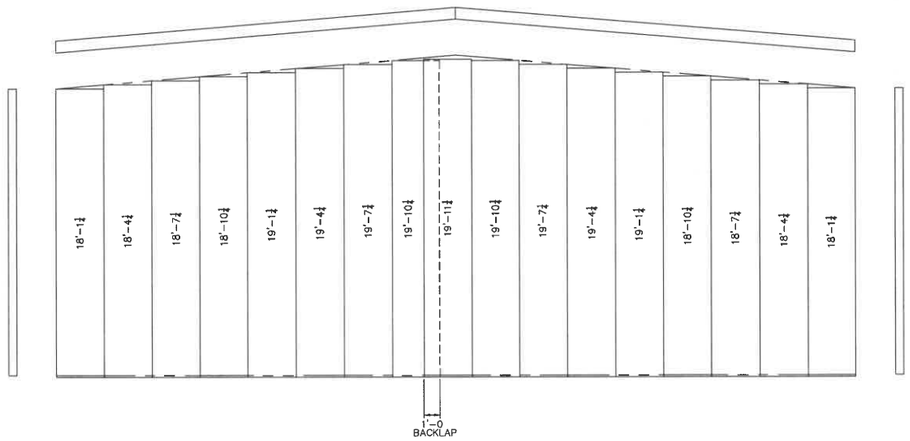
STATE OF ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

SPLICE BOLT TABLE				
CONN.	QTY. SIZE	TYPE	HARDENED WASHERS	BEVELED WASHERS
A	(4) ¾ X 1¾	A325 B&N	4	0
B	(4) ¾ X 1¾	A325 B&N	0	0
C	(8) ¾ X 1¾	A325 B&N	0	0
D	(2) ¾ X 1¾	A325 B&N	0	0

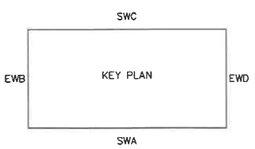


ENDWALL ELEVATION "EWB" AT GRID LINE "1"

PBR WALL PANELS
PANEL COVERAGE = 3'-0
COLOR = ASH GRAY
PANEL PKG. REQ'D. = PBS-1
Field Cut Panel and Trim as required per Construction Details



WALL SHEETING ELEVATION "EWB"
BLDG "A"



MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2023.07.08 12:03:03 -06'00'

ALABAMA LICENSED 21880 PROFESSIONAL ENGINEER M.W. CUSTER

Revision	Date	Description	By	Ck'd

CECO Building Systems

Columbus, MS. (662) 243-6400
Mount Pleasant, IA. (319) 385-8001
Rocky Mount, NC. (252) 977-2101
www.cecobuildings.com

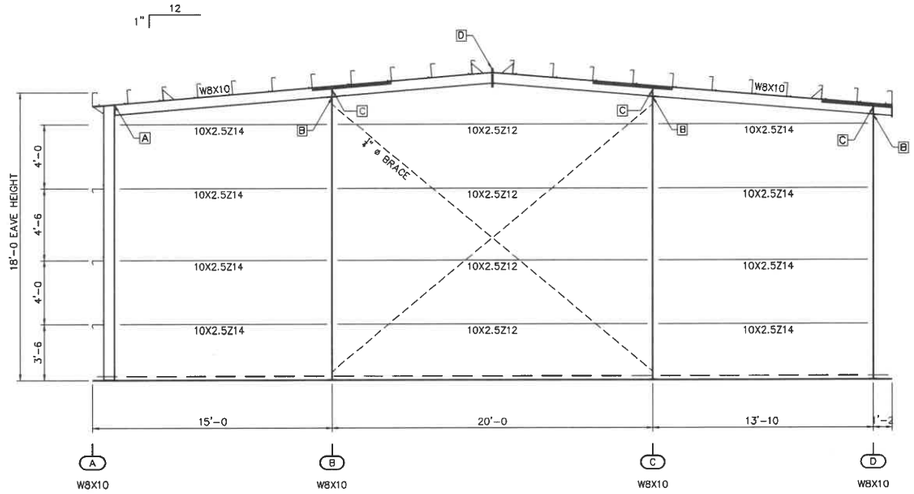
Customer: MAD DASH INC. DBA SOUTHERN
Project Name & Location: MAD DASH INC. DBA SOUTHERN
2210 MAIN ST STE G
DAPHNE, AL 36526-4546 US
Drawing Status:
1912 S YOUNG ST
FAIRHOPE, AL 36532-1619 US
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale:	NOT TO SCALE
Drawn by:	ALN 2/3/23
Checked by:	ABE 2/7/23
Project Engineer:	JDM
Job Number:	19-B-27589-1
Sheet Number:	E6 of 10

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

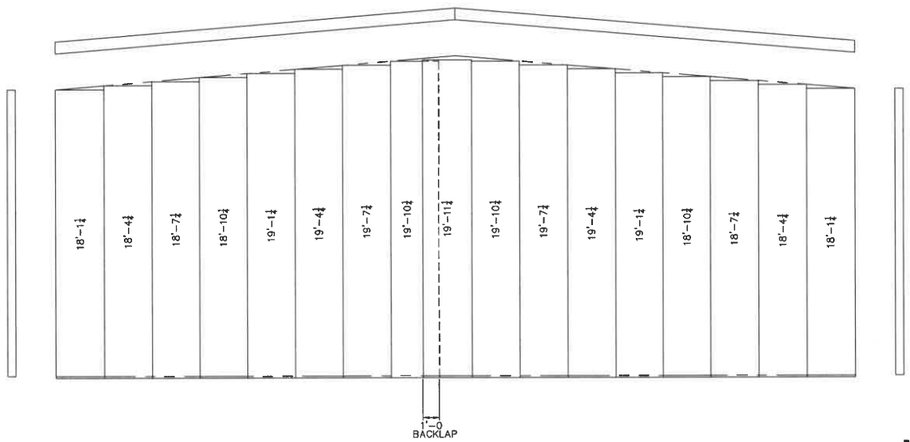
M.W. Custer, P.E.
Alabama P.E. PE21880

SPLICE BOLT TABLE				
CONN.	QTY. SIZE	TYPE	HARDENED WASHERS	BEVELED WASHERS
A	(2) ¾ X 1¾	A325 B&N	0	0
B	(4) ¾ X 1¾	A325 B&N	4	0
C	(4) ¾ X 1¾	A325 B&N	0	0
D	(8) ¾ X 1¾	A325 B&N	0	0

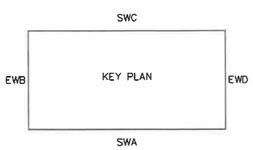


ENDWALL ELEVATION "EWD" AT GRID LINE "6"

PBR WALL PANELS
PANEL COVERAGE = 3'-0
COLOR = ASH GRAY
PANEL PKG. REQ'D. = PBS-3
Field Cut Panel and Trim as required per Construction Details



WALL SHEETING ELEVATION "EWD"
BLDG "A"



MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:03:16 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

Revision	Date	Description	By	Ck'd

CECO Building Systems

Columbus, MS. (662) 243-6400
Mount Pleasant, IA. (319) 385-0001
Rocky Mount, NC. (252) 977-2131
www.cecobuildings.com

Customer: MAD DASH INC. DBA SOUTHERN STEEL
2210 MAIN ST STE G
DAPHNE, AL 36526-4546 US
ATTN: CHRIS WILLIAMS
Drawing Status: ☐ Preliminary (Not For Construction) ☐ For Approval (Not For Construction)

Project Name & Location:
MAD DASH INC. DBA SOUTHERN STEEL
1501 YOUNG ST
FAIRHOPE, AL 36532-1619 US

☒ For Construction Permit
☐ For Erector Installation

Scale:	NOT TO SCALE
Drawn by:	ALN 2/3/23
Checked by:	ABE 2/7/23
Project Engineer:	JDM
Job Number:	19-B-27589-1
Sheet Number:	E7 of 10

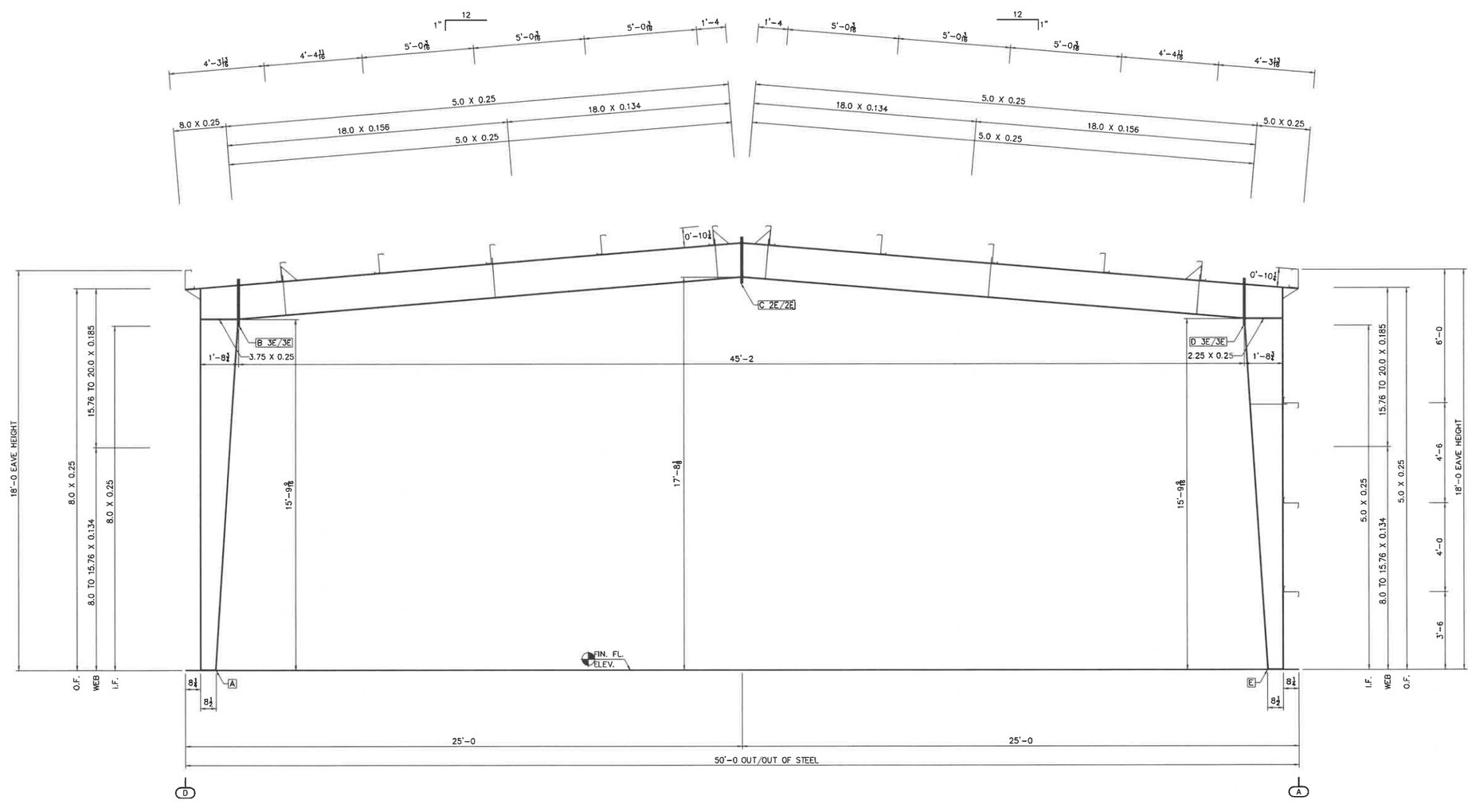
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

M.W. Custer, P.E.
Alabama P.E. PE21880

OY/PROJECTS\XDS-V8-19-02 FRAME = 19-B-27589/ver01-jose.martinezjimenez/BLDG-A/Drftg/x25L 19-B-27589/ver01-jose.martinezjimenez/BLDG-A/Drftg/x25R 01/31/23 16:05:50 01/31/23 16:05:50
OY/PROJECTS\XDS-V8-19-02 FB SET = Eng/19-B-27589/ver01-jose.martinezjimenez/BLDG-A/DRFTG/x25L Eng/19-B-27589/ver01-jose.martinezjimenez/BLDG-A/DRFTG/x25R

GENERAL NOTES
FRAME CLEARANCES SHOWN ARE APPROXIMATE AND MAY VARY DUE TO CONDITIONS (DEFLECTION).

VERTICAL CLEARANCE DIMENSIONS ARE FROM FINISHED FLOOR REFERENCE ELEVATION.



CROSS SECTION AT FRAME LINES "2 THRU 5"

	PLATE SIZE TABLE		SPLICE BOLT TABLE			
CONN.	LOW SIDE	HIGH SIDE	QTY. SIZE	TYPE	HARDENED WASHERS	BEVELED WASHERS
A	8 X 0.375 X 8 1/2					
B	8 X 0.5 X 2'-1 5/16	6 X 0.5 X 2'-0 13/16	(12) 3/4 X 2"	A325 B&N	0	0
C	6 X 0.5 X 2'-0 7/16	6 X 0.5 X 2'-0 13/16	(8) 3/4 X 2"	A325 B&N	0	0
D	6 X 0.5 X 2'-1 5/16	6 X 0.5 X 2'-0 13/16	(12) 3/4 X 2"	A325 B&N	0	0
E	6 X 0.375 X 8 1/2					

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:03:27 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

Revision	Date	Description	By	Ck'd

CECO Building Systems
Columbus, MS. (662) 245-5400
Mount Pleasant, IA. (319) 385-5001
Rocky Mount, NC. (252) 977-2101
www.cecobuildings.com

Project Name & Location:
MAD DASH INC. DBA SOUTHERN STEEL, LLC
19121 YOUNG ST
FAIRHOPE, AL 36532-1619 US

Customer:
MAD DASH INC. DBA SOUTHERN STEEL, LLC
2210 MAIN ST STE G
DAPHNE, AL 36526-4946 US

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale: NOT TO SCALE
Drawn by: ALN 2/3/23
Checked by: ABE 2/7/23
Project Engineer: JDM
Job Number: 19-B-27589-1
Sheet Number: E8 of 10

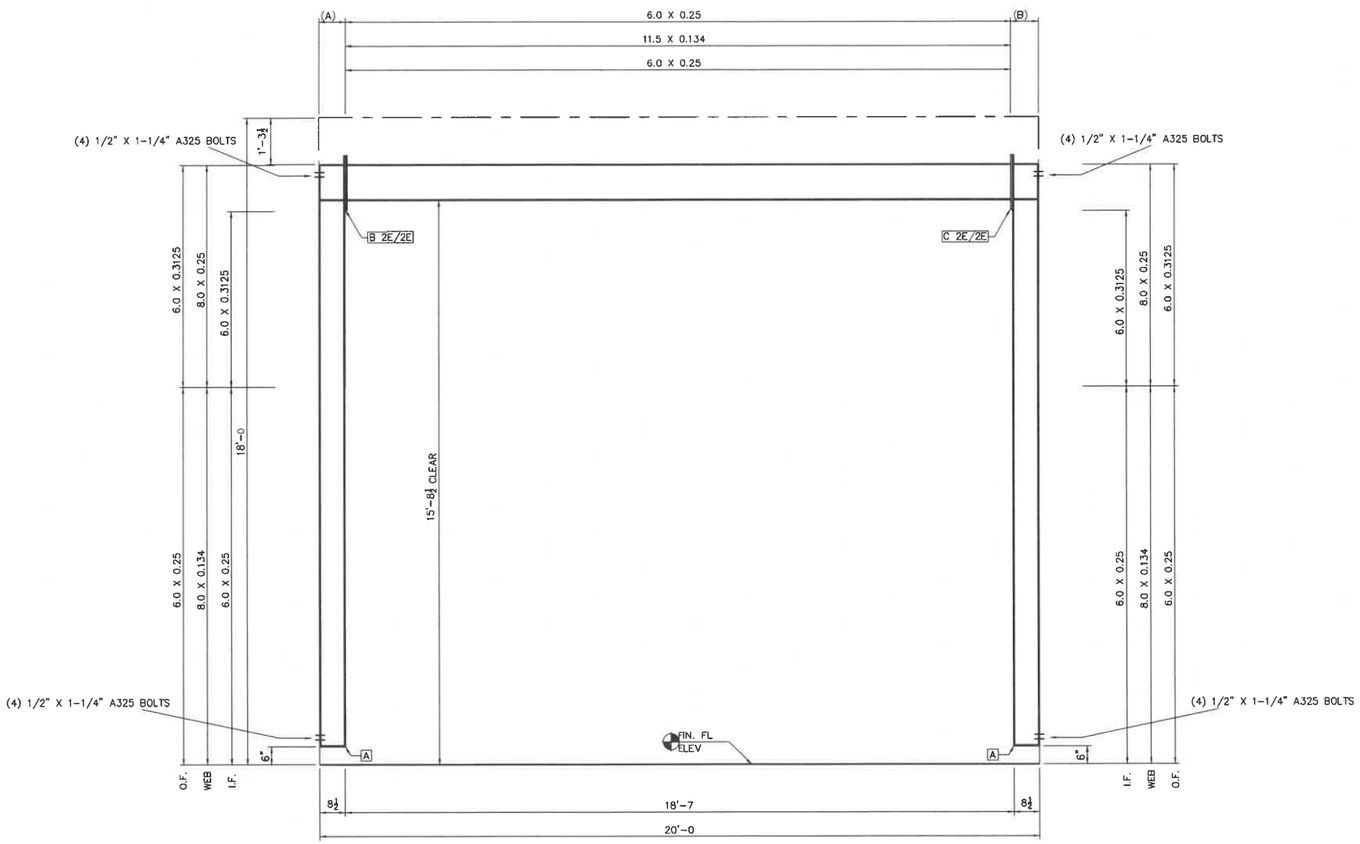
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

M.W. Custer, P.E.
Alabama P.E. PE21880

OY/PROJECTS\XDS-V8-19-02 FRAME = 19-B-27589/ver01-jose.martinezjimenez/BLDG-A/Drftg/x03L 01/31/23 16:06:59

GENERAL NOTES
FRAME CLEARANCES SHOWN ARE APPROXIMATE AND MAY VARY DUE TO CONDITIONS (DEFLECTION).

VERTICAL CLEARANCE DIMENSIONS ARE FROM FINISHED FLOOR REFERENCE ELEVATION.



CROSS SECTION AT PORTAL FRAME
AT GRID LINE "D"

PLATE SIZES
(A) 6.0 X 0.25
(B) 6.0 X 0.25

	PLATE SIZE TABLE		SPLICE BOLT TABLE			
CONN.	LOW SIDE	HIGH SIDE	QTY. SIZE	TYPE	HARDENED WASHERS	BEVELED WASHERS
A	6" X 0.375 X 8½					
B	6" X 0.5 X 1'-6⅛	6" X 0.5 X 1'-6⅛	(8) ¾ X 2"	A325 B&N	0	0
C	6" X 0.5 X 1'-6⅛	6" X 0.5 X 1'-6⅛	(8) ¾ X 2"	A325 B&N	0	0

Revision	Date	Description	By	Ck'd

CECO Building Systems

Columbus, MS: (662) 243-8400
Mount Pleasant, IA: (319) 385-0001
Rocky Mount, NC: (252) 977-2101
www.cecobuildings.com

Customer:
MAD DASH INC. DBA SOUTHERN TEE'S
2210 MAIN ST STE G
DAPHNE, AL 36526-4546 US
ATTN: CHRIS JUDY

Project Name & Location:
MAD DASH INC. DBA SOUTHERN
19121 YOUNG ST
FAIRHOPE, AL 36532-1619 US

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale:	NOT TO SCALE
Drawn by:	ALN 2/3/23
Checked by:	ABE 2/7/23
Project Engineer:	JDM
Job Number:	19-B-27589-1
Sheet Number:	E9 of 10

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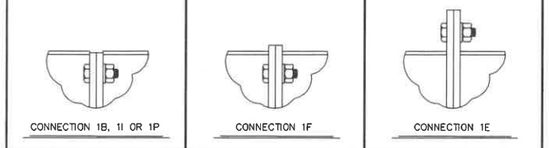
M.W. Custer, P.E.
Alabama P.E. PE21880

MICHAEL W CUSTER

Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:03:40 -06'00'

ALABAMA LICENSED PE 21880 PROFESSIONAL ENGINEER M.W. CUSTER

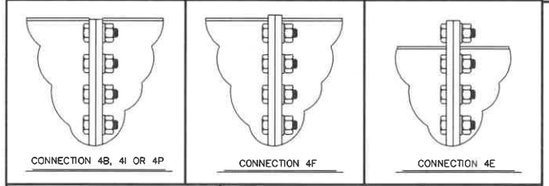
CONNECTION CODES
(FOR TOP AND BOTTOM BOLT PATTERN)



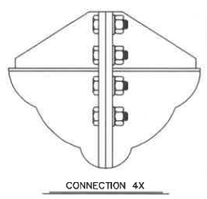
CONNECTION 1B, 1I OR 1P | CONNECTION 1F | CONNECTION 1E

CONNECTION 2B, 2I OR 2P | CONNECTION 2F | CONNECTION 2E

CONNECTION 3B, 3I OR 3P | CONNECTION 3F | CONNECTION 3E



CONNECTION 4B, 4I OR 4P | CONNECTION 4F | CONNECTION 4E



CONNECTION 4X

4 E /2 E H
- CONNECTION DESIGNATION
- BLANK = STANDARD CONNECTION; H = HEAVY CONNECTION
- BOTTOM CONNECTION CODE
- BOTTOM QUANTITY OF BOLT ROWS
- CONNECTION DESIGNATION
- BLANK = STANDARD CONNECTION; H = HEAVY CONNECTION
- TOP CONNECTION CODE
- TOP QUANTITY OF BOLT ROWS

CONNECTION CODE FORMAT

CONNECTION CODE DESCRIPTION

B = THIS DESCRIPTION CODE IS USED TO DEFINE SHEAR CONNECTIONS. BOLTS ARE LOCATED INSIDE THE TOP FLANGE AND CONNECTION PLATE IS RECESSED 1/8" BELOW THE TOP FLANGE. CONNECTION PLATE LENGTH MUST BE A MINIMUM OF HALF THE RAFTER WEB DEPTH AND SHALL NOT EXCEED THE RAFTER TOTAL DEPTH.

E = THIS DESCRIPTION CODE IS USED TO DEFINE MOMENT CONNECTIONS. BOLTS ARE LOCATED WITH ONE SET OUTSIDE THE TOP OR BOTTOM FLANGE AND THE REMAINING SETS ARE LOCATED INSIDE THE TOP OR BOTTOM FLANGE.

F = THIS DESCRIPTION CODE IS USED TO DEFINE MOMENT CONNECTIONS. BOLTS ARE LOCATED INSIDE THE TOP OR BOTTOM FLANGE AND CONNECTION PLATE PROJECTS 1/2" BEYOND THE TOP OR BOTTOM FLANGE.

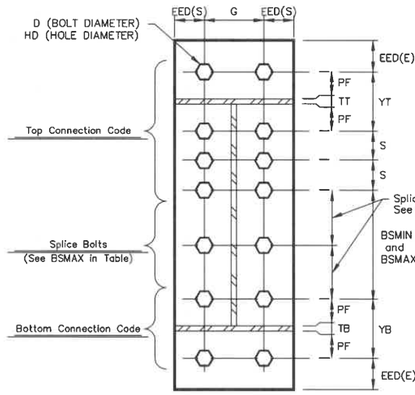
I = THIS DESCRIPTION CODE IS USED TO DEFINE MOMENT CONNECTIONS. BOLTS ARE LOCATED INSIDE THE TOP OR BOTTOM FLANGE AND CONNECTION PLATE IS RECESSED 1/8" BELOW THE TOP OR BOTTOM FLANGE.

P = THIS DESCRIPTION CODE IS USED TO DEFINE SHEAR CONNECTIONS. BOLTS ARE LOCATED INSIDE THE TOP FLANGE AND CONNECTION PLATE IS RECESSED 1/8" BELOW THE TOP FLANGE. CONNECTION PLATE LENGTH MUST BE A MINIMUM OF HALF THE RAFTER WEB DEPTH AND SHALL NOT EXCEED THE RAFTER TOTAL DEPTH.

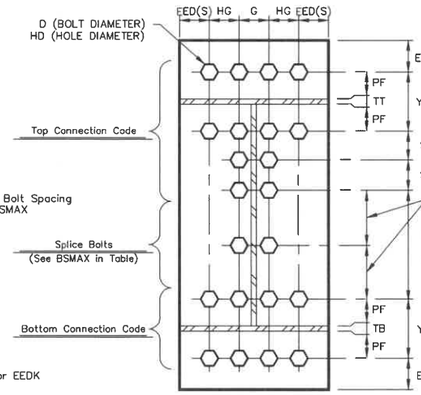
4X = THIS DESCRIPTION CODE IS USED TO DEFINE MOMENT CONNECTIONS. BOLTS ARE LOCATED WITH TWO SETS EACH SIDE OF THE TOP OR BOTTOM FLANGE WITH A GUSSET PLATE OUTSIDE THE TOP AND BOTTOM FLANGE OR COLUMN CAP PLATE.

CONNECTION BOLT DATA

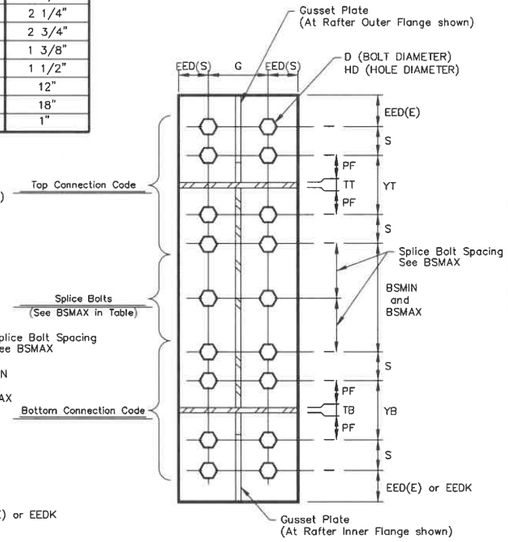
NAME	DESCRIPTION FOR A325 BOLT DIMENSIONS	A325 CONNECTION BOLT DIMENSIONS					
D	DIAMETER OF THE BOLT	1/2"	3/4"	7/8"	1"	1 1/4"	1 1/2"
HD	BOLT HOLE DIAMETER	9/16"	13/16"	15/16"	1 1/16"	1 5/16"	1 9/16"
G	BOLT GAUGE	2 1/2"	3"	4"	3 1/2"	4"	5 1/2"
	MAX. WEB THICKNESS (Max. 5/16" Fillet Weld) WITHOUT WASHER	1"	1 1/8"	1 7/8"	1 1/4"	1 3/8"	2 1/8"
	MAX. WEB THICKNESS (Max. 5/16" Fillet Weld) WITH WASHER	3/4"	7/8"	1 5/8"	7/8"	7/8"	1 7/8"
HG	HEAVY CONN. BOLT GAUGE	N/A	2 1/4"	2 5/8"	3"	3 3/4"	4"
S	NORMAL BOLT SPACING	2 1/2"	3"	3 1/4"	3 1/2"	4"	4 1/2"
BSMIN	MINIMUM SPACING BETWEEN TOP & BOTTOM SETS OF BOLTS	1 1/2"	2 1/4"	2 5/8"	3"	3 3/4"	4"
BSMAX	MAXIMUM BOLT SPACING BETWEEN TOP AND BOTTOM SETS OF BOLTS ON CONNECTION PLATES LESS THAN OR EQUAL TO 3/4" THICK	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"
		SPLICE BOLT SPACING (NOT TO EXCEED 2'-0)		1/2 BSMAX (±1/16") WHEN BSMAX = 2'-0 1/16" TO 4'-0 1/3 BSMAX (±1/16") WHEN BSMAX = 4'-0 1/16" TO 6'-0 1/4 BSMAX (±1/16") WHEN BSMAX = 6'-0 1/16" TO 8'-0		
BFCD	MINIMUM BOLT-TO-FLANGE CLEARANCE AT OUT OF NUT SEE BOLT AT FLANGE DETAIL	1 1/2"	1 3/4"	1 7/8"	2 1/4"	2 1/2"	2 3/4"
PF	MINIMUM BOLT-TO-FLANGE CLEARANCE AT CONNECTION PLATE SEE BOLT AT FLANGE DETAIL	(BFCD + RNWT) PF INSIDE OF FLANGE IS INCREASED BASED ON THE YT & YB VALUE. PF FOR CONNECTION B, F, I AND P ARE THE SAME AS USED ON CONNECTION E					
NWT	NUT AND WASHER THICKNESS	SEE BOLT AT FLANGE DETAIL. NUT THICKNESS IS EQUAL TO THE BOLT DIAMETER AND .15625" WASHER THICKNESS IS USED EVEN IF A WASHER IS NOT REQUIRED.					
RNWT	RISE ON NUT AND WASHER THICKNESS						
TT	THICKNESS TOP FLANGE	REFER TO FRAME CROSS SECTION DRAWING FOR LARGEST FLANGE THICKNESS EITHER SIDE OF THE CONNECTION.					
TB	THICKNESS BOTTOM FLANGE						
YT	BOLT SPACING TOP (ROUND UP TO NEXT 1/2", MIN = S)	3" + TT	3 1/2" + TT	3 3/4" + TT	4 1/2" + TT	5" + TT	5 1/2" + TT
YB	BOLT SPACING BOTTOM (ROUND UP TO NEXT 1/2", MIN = S)	or TB Sloped	or TB Sloped	or TB Sloped	or TB Sloped	or TB Sloped	or TB Sloped
EED(E)	MINIMUM END EDGE DIMENSION	1 1/4"	1 1/4"	1 1/2"	1 3/4"	2 1/4"	2 5/8"
EED(S)	MINIMUM SIDE EDGE DIMENSION	3/4"	1"	1 1/8"	1 1/4"	1 5/8"	2 1/4"
EEDK	END EDGE DIMENSION AT KNEE CONNECTION	1 3/8"	1 3/8"	1 5/8"	1 7/8"	2 3/8"	2 3/4"
BCWM	MINIMUM BOLT CLEARANCE FROM A FLANGE OR WEB WELD — WITHOUT WASHER	7/16"	5/8"	3/4"	13/16"	1"	1 3/8"
	MINIMUM BOLT CLEARANCE FROM A FLANGE OR WEB WELD — WITH HARDENED WASHER	9/16"	3/4"	7/8"	1"	1 1/4"	1 1/2"
WCSM	MINIMUM WIDTH OF CONNECTION PLATE (Standard Connection)	5"	6"	8"	8"	10"	12"
WCHM	MINIMUM WIDTH OF CONNECTION PLATE (Heavy Connection)	N/A	10"	12"	12"	16"	18"
TCMIN	MINIMUM THICKNESS OF CONNECTION PLATE	1/4"	3/8"	7/16"	1/2"	5/8"	1"



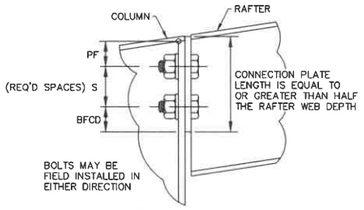
STANDARD CONNECTION DESIGNATION
(CODE 4E/2E SHOWN)



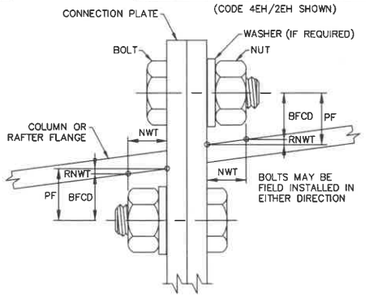
HEAVY CONNECTION DESIGNATION
(CODE 4EH/2EH SHOWN)



4X CONNECTION DESIGNATION
(CODE 4X/4X SHOWN)



CONNECTION B & P
(Low Side Shown, High Side Similar)



BOLT AT FLANGE DETAIL
(Top Flange Shown, Bottom Flange Similar)

Frame Documentation
A325 Connection Bolt Details

Page 05-12-10
Date Jun '18 Rev. 04

B 4E/2EH
- Connection Code (See "Connection Code Format" on this drawing)
- Connection Location

CROSS SECTION CONNECTION CODE KEY
(AS SHOWN AT CONNECTIONS ON FRAME CROSS SECTION DRAWINGS)

Flange Brace Material Schedule	
Part Mark	Material
FB4_	L 2" x 2" x 14 Ga.
FB5_	L 2" x 2" x 14 Ga.
FB6_	L 2" x 2" x 1/8"
FB7_	L 2 1/2" x 2 1/2" x 5/16"

Ck'd	By	Description	Date	Revision

CECO Building Systems
Columbus, MS (662) 243-8400
Mount Pleasant, IA (319) 385-8001
Rocky Mount, NC (252) 977-2151
www.cecobuildings.com

Project Name & Location:
MAD DASH INC. DBA SOUTHERN STEEL
19121 YOUNG ST
FAIRHOPE, AL 36532-1619 US

Customer:
MAD DASH INC. DBA SOUTHERN STEEL
2210 MAIN ST STE G
LAWRENCEVILLE, GA 30046 US
ATTN: CORNELIUS LINDA

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale:	NOT TO SCALE
Drawn by:	ALN 2/3/23
Checked by:	ABE 2/7/23
Project Engineer:	JDM
Job Number:	19-B-27589-1
Sheet Number:	E10 of 10

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

M.W. Custer, P.E.
Alabama P.E. PE21880

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:03:53 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

Field Service Procedures

In Order To Give You Prompt Services And Keep Problems To A Minimum, Please Handle Any Shortages Or Back Charges In The Following Manner:

1. Carefully Check Your Packing List While Unloading.
2. Mark Any Items Which Appear To Be Missing And Notify The Field Service Department At The Number Shown In The Title Block As Soon As Possible. Calling Someone Else Could Delay The Proper Response.

INITIAL CLAIM:
In The Event Of An Error, The Customer Must Promptly Make A Written Or Verbal "Initial Claim" to The Manufacturer For The Correction Of Design, Drafting, Bill Of Materials Or Fabrication Error.
The "Initial Claim" Includes:

1. Description Of The Nature And Extent Of The Errors, Including Quantities.
2. Description Of The Nature And Extent Of Proposed Corrective Work, Including Estimated Man-Hours.
3. Materials To Be Purchased From Other Than the Manufacturer, Including Estimated Quantities and Cost.
4. Maximum Total Cost Of Proposed Corrective Work And Materials To Be Purchased From Other Than The Manufacturer.

SHORT MATERIALS:
Immediately Upon Delivery Of Materials, Quantities Are To Be Verified By The Customer Against Quantities That Are Billed On The Shipping Documents. Neither The Manufacturer Nor The Carrier Is Responsible For Material Shortages Against The Quantities Billed On The Shipping Documents If Such Shortages Are Not Noted On The Shipping Documents When The Material Is Delivered And Acknowledged By The Carrier's Agent. If The Carrier Is The Manufacturer, Claims For Shortages Are To Be Made By The Customer To The Common Carrier. If The Material Quantities Received Are Correct According To The Quantities Billed On The Shipping Documents, But Are Less Than The Quantities Ordered Or The Quantities That Are Necessary To Complete The Metal Building According To The Order Documents, Claim Is To Be Made To The Manufacturer.

DAMAGED OR DEFECTIVE MATERIAL:
Damaged Or Defective Material, Regardless Of The Degree Of Damage, Must be Noted On The Shipping Documents By The Customer And Acknowledged By The Carrier's Agent. The Manufacturer Is Not Responsible For Material Damaged In Unloading Of Packages Or Nested Materials, Including, But Not Limited To: Fasteners, Sheet Metal, "C" And "Z" Sections And Covering Panels That Become Wet And/Or Damaged By Water While In The Possession Of Others. Packaged Or Nested Material That Become Wet In Transit Must Be Unpacked, Unstacked And Dried By The Customer. If The Carrier Is The Manufacturer, The Customer Must Make Claim For Damaged Directly To The Manufacturer. If The Carrier Is A Common Carrier, The Customer Must Make The Claim For Damage To The Common Carrier. The Manufacturer Is Not Liable For Any Claim Whatsoever Including, But Not Limited To Labor Charges Of Consequential Damages Resulting From Customer's Use Of Damaged Or Defective Materials That Can Be Detected By Visual Inspection.

EXCESSIVE MATERIAL:
The Manufacturer Reserves The Right To Recover Any Material Delivered In Excess Of Those Required By The Order Documents.

OIL CANNING IS NOT A CAUSE FOR REJECTION

Authorization For Corrective Work

Normal Erection Operations Include The Correction Of Minor Misfits By Amounts Of Reaming, Chipping, Welding Or Cutting And The Drawing Of Elements Into Line Through The Use Of Drift Pins. Errors That Cannot Be Corrected By The Foregoing Means Or Which Require Major Changes In The Member Configuration Should Be Reported Immediately To The Owner And The Fabricator By The Erector, To Enable Whoever Is Responsible Either To Correct The Error Or Approve The Most Efficient And Economical Method Of Correction To Be Used By Others. (AISC 303-10, Section 7.14). If The Error Is The Fault Of The Manufacturer An "Authorization For Corrective Work" Must Be Issued In Writing By The Manufacturer To Authorize The Corrective Work At A Cost Not To Exceed The Maximum Total Cost Set Forth. Alternative Corrective Work Other Than That Proposed In The "Initial Claim" May Be Directed By The Manufacturer In The "Authorization Of Corrective Work". Only The Field Service Department May Authorize Corrective Work.

FINAL CLAIM:
The "Final Claim" In Writing Must Be Forwarded By The Customer To The Manufacturer Within (10) Days Of The Completion Of The Corrective Work Authorized By The Manufacturer.

THE "FINAL CLAIM" MUST INCLUDE:

1. Actual Number Of Man-Hours By Dated Of Direct Labor Use On Corrective Work And Actual Hourly Rate Of Pay.
2. Taxes And Insurance On Total Actual Direct Labor.
3. Other Direct Costs On Actual Direct Labor.
4. Cost Of Materials (Not Minor Supplies) Authorized By The Manufacturer To Be Purchased From Other Than The Manufacturer, Including Copies Of Paid Invoices.
5. Total Actual Direct Cost Of Corrective Work (Sum Of 1, 2, 3, And 4). The "Final Claims Are Credited To The Customer By The Manufacturer In The Amount Not To Exceed The Lesser Of The Maximum Total Cost Set Forth In The "Authorization For Corrective Work" Or The Total Direct Cost Of Corrective Work.

** IMPORTANT NOTE **
Cost Of Equipment (Rental Or Depreciation), Small Tools, Supervision, Overhead And Profit Are Not Subjected To Claims.

SHIPMENT ARRIVAL TIME:
Every Effort Will Be Made To See That The Carrier Arrives At The Jobsite On The Requested Hour. Manufacturer Makes No Warranty And Accepts No Responsibility For Costs Associated With A Shipment Not Arriving At The Requested Time Unless A Separate Agreement Has Been Made In Writing For A Guaranteed Arrival Time.

Unloading, Handling And Storage

STRUCTURAL:
A Great Amount Of Time And Trouble Can Be Saved If The Building Parts Are Unloaded At The Building Site According To A Pre-Arranged Plan. Proper Location And Handling Of Components Will Eliminate Unnecessary Handling.

NOTE:
Piece Marks Are Stenciled On The Primary Structural Members At The Lower End, 1'-0" From The End. Inspect All Shipments Prior To Releasing The Tie-downs For Loads That May Have Shifted During Transit.

REMEMBER SAFETY FIRST:
Blocking Under Columns And Rafters Protect The Splice Plates And The Slab From Damage During The Unloading Process. It Also Facilitates The Placing Of Slings And Cables Around Members For Later Lifting And Allows Members To Be Bolted Together Into Sub-assemblies While On The Ground. Extra Care Should Always Be Exercised In The Unloading Operation To Prevent Injuries From Handling Steel And To Prevent Damage To Materials And The Concrete Slab. If Water Is Allowed To Remain For Extended Periods In Bundles Of Primed Parts Such As Girts, Purlins, Etc., The Pigment Will Fade And The Paint Will Gradually Soften Reducing Its Bond To The Steel. Therefore, Upon Receipt Of A Job, All Bundles Of Primed Parts Should Be Stored At An Angle To Allow Any Trapped Water To Drain Away And Permit Air Circulation For Drying. Puddles Of Water Should Not Be Allowed To Collect And Remain On Columns Or Rafters For Same Reason.

The Coat Of Shop Primer Is Intended To Protect The Steel Framing Only For A Short Period Of Exposure To Ordinary Atmospheric Conditions. The Coat Of Shop Primer Does Not Provide The Uniformity Of Appearance, Or The Durability And Corrosion Resistance Of A Field Applied Finish Coat Of Paint Over Shop Primer.

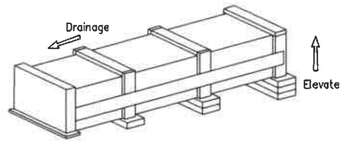
Roof And Wall Panels

Manufacturer's Roof And Wall Panels Include Color Coated, Galvalume, And Galvanized, Provide Excellent Service Under Widely Varied Conditions. All Unloading And Erection Personnel Should Fully Understand That These Panels Are Quality Merchandise, Which Merits Cautious Care And Handling.

UNDER NO CIRCUMSTANCES SHOULD PANELS BE HANDLED ROUGHLY
Packages Of Sheets Should Be Lifted Off The Truck With Extreme Care Taken To Ensure That No Damage Occurs To Ends Of The Sheets Or to Side Ribs. The Packages Should Be Stored Off The Ground Sufficiently High To Allow Air Circulation Underneath The Packages. This Avoids Ground Moisture And Deters People From Walking On The Packages. One End Of The Package Should Be Elevated To Encourage Drainage In Case Of Rain. The Manufacturer Exercises Caution During Fabrication An Shipping Operations To Ensure That All Panel Stock Is Kept Dry. However Due To Climatic Conditions, Water Formed By Condensation Of Humid Air Become Trapped Between Sheets. Water Can Also Be Trapped Between The Stacked Sheets When Exposed To Rain. This May Discoloration Caused By Trapped Moisture. The Stain Is Usually Superficial And Has Little Effect On The Appearance Or Service Life Of The Panels As Long As It Not Permitted To Remain On The Panel. However, Moisture In Contact With The Surface Of The panel Over An Extended Period Can Severely Attack The Finish And Reduce The Effective Service Life. See R1-07 Titled "Damage From Condensation Or Trapped Water".

CAUTION:
Care Should Always Be Taken When Walking On Panels. Use Safety Lines And Net When Necessary. Panels Are Slippery, Wipe Dry Any Moisture Or Surface Material That Has Puddle From Bundles Stored On A Slope. Dew, Frost, Or Other Forms Of Moisture Greatly Increase The Slipperiness Of The Panels. Always Assume Panel Surface Is Slippery And Act Accordingly. Never Walk Of Step On Skylights Or Translucent Panels.

Use Wood Blocking To Elevate And Slope The Panels In A Manner That Allows Moisture To Drain. Wood Blocking Placed Between Bundles Will Provide Additional Air Circulation. When Handling Or Uncrating The Panels, Lift Rather Than Slide Them Apart. Burred Edges May Scratch The Coated Surfaces When Sheets Are Slid Over One Another. Never Allow Panels To Be Walked On While On The Ground.



Roof And Wall Panel Damage During Construction

The Quality Of Workmanship In Steel Construction Practices And Handling Methods Used During The Construction Of The Metal Building Can Significantly Affect The Appearance And Performance Of The Building Panels. Panel Damage During Construction Can Be The Result Of Faulty Installation Methods And/or Carelessness.

Overdriven Fasteners Cause Indentations Or Shallow Pockets In The Panel Around The Fastener Head. Rain Water Or Condensation Moisture Combined With Atmospheric Pollutants (principally Sulfur Dioxides) And Dirt Particles Collect In These Pockets. The Combination Of Pollutants And Water Creates Acid Solutions That Will Cause Corrosion Damage To The Panel And Fastener. Rain May Wash Some Pollutants Away, But Moisture In Form Of High Humidity Can Keep These Areas Wet And Continue The Problem. Overdriving The Fastener Also Forces The Sealing Washer From Under The Head Creating A Leak At This Point. Proper Torque Adjustment Of The Screw Gun Or Preferably The Use Of A Depth Gauge Will Eliminate The Problem Of Overdriven Fasteners.

It Is Extremely Important That All Drill Shavings From The Installation Of Panel Fasteners And Fillings From The Saw Cutting Of Panels Be Removed From The Panel Surface. Corrosion Can Occur In A Matter Of Hours When These Shavings Or Fillings Are Not Removed And Are In Contact With Water Or Condensed Moisture. When Panels Are Pre-Drilled Or Cut In The Stack Prior To Erection All Shavings Must Be Cleaned From Both Sides Of The Panel To Prevent Corrosion Of The Panel By These Particles. It Is Imperative That The Roof Be Swept Clean At Least Daily And Certainly At Job Completion. The Final Cleaning Of The Roof Should Be Done Prior To Installing The Gutter So That The Shavings Are Not Deposited Into The Gutter And Left To Corrode. Any Other Foreign Objects Or Debris Left By Construction Personnel Should Also Be Removed From The Roof During The Erection Of The Roof And The Installation Of Such Equipment As Air Condition Units, Etc..

Personnel Walking On The Panel Can Cause Damage. Workmen Should Step Or Walk In The Broad Flat Areas Of The Panel And Avoid Stepping On The Panel Ends And Edges Which Can Be Bent By Careless Handling. If This Damage Is Severe, The Edges Must Be Straighten Prior To Erection Since The Appearance And/or Weather Tightness Of The Panel Could Be Affected. Dragging One Panel Across Another Can Cut Or Abrade The Coating Causing Unsightly Marks On The Panel Surface.

Attempts To Erect Panels During Windy Conditions Should Be Avoided To Prevent Damage And Of Safety Considerations.

Leaving Dirt Piled Against The Exterior Wall Panels At The Foundation Will Cause Panel Damage. This Dirt May Be Wet Or At Least Contain Some Moisture. Mud May Have Splashed Onto The Wall During Construction. Corrosion Damage May Occur Where This Dirt Or Mud Contacts The Panel. In Areas Where Lime Stabilization Of The Soil Is Required, Corrosion Damage From The Soil's Content Will Be Accelerated And Most Likely Be Severe. All Dirt Must Be Removed From The Panel Walls At The Time Of Completion Of Work. Pre-Painted Panels May Require Touch-up If The Coating Has Been Damaged During Handling Or Erection.

The Appearance Of The Building May Be Affected If Damaged Spots Or Scratches Are Located In Highly Visible Places Such As Around Doors, Windows, Etc.. If Damage Is Extensive Then Replacement Of The Entire Panel Should Be Considered.

Types Of Finishes

SHOP PRIMED STEEL:
All Structural Members Of The Metal Building System Not Fabricated Of Corrosion Resistant Material Or Protected By A Corrosion Resistant Coating Are Painted With One Coat Of Shop Primer Meeting The Performance Requirements Of SSPC Paint Specification No.15. The Coat Of Shop Primer Is Intended To Protect The Steel Framing For Only A Short Period Of Exposure To Ordinary Atmospheric Conditions. Shop Primed Steel Which Is Stored In The Field Pending Erection Should Be Kept Free Of The Ground And So Positioned As To Minimize Water Holding Pockets, Dust, Mud And Other Contamination Of The Primer Film. Repairs Of Damaged To Primed Surfaces And/Or Removal Of Foreign Material Due To Improper Field Storage Or Site Conditions Are Not The Responsibility Of The Manufacturer. The Manufacturer Is Not Responsible For Deterioration Of The Shop Coat Of Primer Or Corrosion That May Result From Exposure To Atmospheric And Environmental Conditions, Nor The Compatibility Of The Primer To Any Field Applied Coating. Minor Abrasions To The Shop Coat (Including Galvanizing) Caused By Handling, Loading, Shipping, Unloading And Erection After Painting Or Galvanizing Are Unavoidable. (MBMA 2012, Chapter IV 4.2.4).

GALVALUME:
Galvalume Is The Trade Name For A Patented Steel Sheet And Coil Product Having A Coating Of Corrosion Resistant Aluminum-Zinc Alloy. The Mixture Is Balanced To Obtain The Coating That Retains The Corrosion Resistance And Heat Reflectivity Of Aluminum And Galvanic Protection Of Zinc. The Best Properties Of Both Aluminum And Zinc Are Combined In This Coating And Offer Added Service Life For The Building.

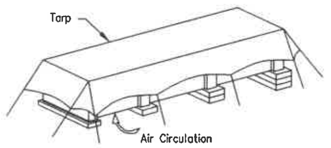
Pre-Painted:
Using Galvalume Steel As A Substrate, Pre-Painted Steel Is Given An Additional Rust Inhibitor Primer Coat. This Primer Coat Further Increases The Corrosion Resistance. These Coatings Are Applied To The Exterior Surface Of The Panels And A Wash Coat Designed Only For Interior Use, Is Applied On The Opposite Side. Galvalume And Pre-Painted Steel Can Give Excellent Service For Many Years If A Few Rules Concerning Their Care And Maintenance Are Observed. All Of These Finishes Are Equally Subject To Damage And Corrosion When Care Is Not Provided.

PAINT AND COATING MAINTENANCE:
Remove Smudge Marks From Bare Galvalume:
Formula 409 Has Proven To Be Somewhat Effective. Lightly Rub With A Clean Cloth And Rinse With Water. Do Not Rub More Than Required To Remove Smudge Marks. No Product Will Remove All Smudge Marks.
Remove Rust Stains:
Soft Scrub Without Bleach Has Proven To be Somewhat Effective. Rub With A Soft Cloth And Rinse With Water. Do Not Rub More Than Required To Remove Stain. No Product Will Completely Remove Rust Stains.
To Touch-Up Scratches In Paint (Not Bare Metal):
Clean Area To Be Painted With Mild Detergent. Rinse Thoroughly And Dry. Using A Small Artist's Brush, Lightly Apply A Minimal Amount Of Color Matched Touch-Up Pain Required To Fill/Cover The Scratch. Contact The Building Manufacturer For Assistance With Ordering/Purchasing Touch-Up Paint As Needed.

Damage From Condensation Or Trapped Water

It Is Extremely Important That The Panels Be Monitored For Evidence Or Trapped Water Or Moisture Condensation While Awaiting Erection. High Humidity Conditions With Temperature Cycling Will Cause Condensation Between Panels Within The Bundle. Condensation Can Occur Frequently Near The Sea Coast Or Other Large Bodies Of Water.

If Jobsite Covers Are Used, They Should Be Tied Away From The Bundle At Corners To Allow Air Circulation Around The Bundle. This Will Help Prevent Moisture Evaporating From The Ground Or Building Floor From Condensing On The Panels. Plastic Or Other Impermeable Covers Are Not Recommended. Immediate Action Is Required If The Panels Are Found To Be Wet From Any Cause. The Bundles Must Be Opened And Each Panel Un-Stacked And Thoroughly Dried On Both Sides. Re-Stacking The Panel At A Slight Angle To Each Other To Prevent Nesting Will Allow Air Circulation And Assist In Keeping The Panel Dry. In Severe Conditions Large Fans Can Be Used To Circulate Air Between The Un-Stacked Panels And Accelerate Drying. Damage To The panel Coating Occurs When Panels Become Wet And Are Allowed To stay wet. damage Can Occur To Nested Panels Within 24 to 48 Hours. This Damage Shows Corrosion And Discoloration Of The Panel Surface And Is Commonly Called Wet Storage. Stain, Zinc Oxidation, Or "White Rust".



A Softening Of The Paint Film Can Occur With Pre-Painted Steel Under Wet Storage Conditions And The Durability Of The Panel Finish Substantially Decrease. Bare Galvanized And Galvalume Panels React More Quickly To Surface Oxidation Since They Lack The Additional Protection Of Paint. Zinc Coated Or Galvalume Panels Under Normal Exposure Form A Zinc Aluminum Oxide Film On Their Surface Allowing A Slow Oxidation Process Called "Weathering" To Occur That Inhibits Further Corrosion. In Nested Bundles Constant Contact Of The Panels With Condensed Or Trapped Water Prevents This Weathering Process.

Rapid Oxidation Of The Zinc or Zinc Aluminum Coating Can Now Occur And May Lead To "Red Rust" In A Short Time. If Discoloration Or Stains Are Minor A Household Cleaner Of The Type Used On Porcelain Sinks And Bathtubs May Be Used To Remove Stains. Wire Brushing Or Abrasive Materials Should be Avoided Since Scratching Or Removal Of The Coating Could Occur. Panel With Significant Damage Should Be Replaced By The Buyer Prior To Erection.

Safety Commitment

The Builder/Contractor Is Responsible For Applying And Observing All Pertinent Safety Rules And OSHA Standards As Applicable.

The Building Manufacturer Has A Commitment To Manufacture Quality Building Components That Can Be Safely Erected. However The Safety Commitment And Job Site Practices Of The Erector Are Beyond The Control Of The Building Manufacturer.

It Is Strongly Recommended That Safe Working Conditions And Accident Prevention Practices Be The Top Priority Of Any Job Site.

Local, State And Federal Safety And health Standards, Whether Standard Statuary Or Customary, Should Always Be Followed To Help Ensure Worker Safety.

Make Sure All Employees Know The Safest And Most Productive Way Of Erecting A Building. Emergency Procedures Should Be Known To All Employees. Daily Meetings Highlighting Safety Procedures Are Also Recommended. The Use Of Hard Hats, Rubber Sole Shoes For Roof Work, Proper Equipment For Handling Material And Safety Nets Where Applicable Are Recommended

For The Purposes Of Determining Lift Requirements, No Bundle Supplied By The Manufacturer Will Exceed 4,000 Pounds. For Further Information Also reference The Bill Of Materials For Individual Member Weights Of Structural Members. If Additional Information Is Required Contact The Field Service Department.

ICE AND SNOW REMOVAL:
Excessive Ice And Snow Removal Should Be Removed From The Roof Immediately To Prevent Damage To Roof And Possible Collapse. Do Not Use Metal Tools To remove The Ice Or Snow As This Can Damage The Paint And/Or Galvalume Coatings. Also Be Careful Around Pipes And Flashing's.
Be Extremely Careful If Your Roof Has Light Transmitting Panels. These Panels Will Not Support A Person's Weight And Will Be Difficult Or Impossible To See If They Are Covered With Ice Or Snow. See MBMA Low-Rise Building Systems Manual, Appendix A8 For Details On Snow Removal Procedures. These Procedures Should Commence When Half Of The Design Roof Snow Load Is Realized.

DEBRIS REMOVAL:
Any Foreign Debris Such As Sawdust,Dirt, Leaves, Animal Droppings, Etc. Will Cause Corrosion Of The Roof, Gutters, Trim, Etc. If Left On The Building Surface For A Long Enough Time. The Roof Should Be Periodically Inspected For Such Conditions And If Found, They Should Be Rectified In A Manner Consistent With These Roof Maintenance Guidelines. Never Allow Treated Lumber Or Concrete/Mortar/Grout To Come In Contact With Roof Panels, Especially Galvalume For Extended Periods Of Time.

PERIODIC INSPECTION:
All High-Strength Shall Be Periodically Be Inspected For Tightness. Particularly In Crane Buildings And After Seismic Or Wind Activity. The Crane Manufacturer Will Specify A Minimum Period But It Should Not Exceed Two Years.

DRAINAGE:

1. Keep Roof Free Of Debris And Keep Debris Out Of Gutter To Allow Water Quickly Drain From The Roof.
2. Do Not Use Wood Blocking To Hold Equipment Off The Panel Seams. This Blocks The Flow Of Water And Hold Moisture.
3. Do Not Allow Rooftop AC Units Or Evaporative Coolers To Drain Onto The Roof.
4. Anything That Traps Or Holds Moisture On A Roof Will Cause Premature Corrosion.

Roof Maintenance Guidelines

1. Inspect Roof For Damage After Heavy Storms.

2. Inspect And Reseal As Necessary All Roof Curbs And Other Penetrations With Urethane Sealant.

3. Always Get Manufacturer Approval Before Making Any Modifications To The Roof.

4. Repaint Any Areas That Are Susceptible To Rust As Required.

5. When Performing Roof Maintenance, Always Take The Following Precautions:
   a. Use Fall Protection And Other Safety Protection As Required.
   b. Do Not Walk On Roof Flashing Such As Gutter, Rake, Hip Or Ridge Flash.
   c. Do Not Walk On Light Transmitting Panels (LTP's). They Will Not Support A Person's Weight.
   d. Guard All LTP's And Roof Openings.
   e. Step Only In The Panel Flat Directly On Or In Close Proximity To A Supporting Roof Structural.

6. After Other Trades Have Been On The Roof For Any Reason, Inspect The Roof For Damage Caused By Workers Including Chemical Or Solvent Spills, Scratches In The Paint Or Galvalume Coating, Excessive Foot Traffic And Punctures. Make Sure That All Debris Or Scrap Left Behind By Workers Is Removed From The Roof Immediately. Avoid Using Cutoff Saws And Welding Equipment Over The Roof. The Roof Must Adequately Protected.

FOOT TRAFFIC:
Keep Foot Traffic To A Minimum. Heavy Foot Traffic Can Cause Ponding On Low Pitched Roofs. This Is Particularly True Just Upslope From The Eave And At Endlaps.
Always Walk In The Flat Of The Panel Near A Supporting Roof Structural. Do Not Walk On Trim Or In Gutters.
On Bare Galvalume Roofs, Excessive Foot Traffic May Cause Black Burnish Marks. If Regular Foot Traffic Is Planned For A Roof, Provisions Should Be Made For A Properly Designed And Installed Walkway System. In Order To Limit Access To The Roof, Roof Hatches Or Access Ladders Should Be Locked At All Times. A Sign Posted At The Access Site Stating That Only Authorized Personnel Are Allowed On The Roof. In Addition A Log Book Should Be Kept Of All Visits To The Roof And The Reason For Such Visits.

DISSIMILAR METALS:
Never Allow Your Roof To Come In Contact With, Or Water Runoff From Any Dissimilar Metal Including But Not Limited To:
Copper, Lead Or Graphite, This Includes Copper And Arsenic Salts Used In Treated Lumber, Calcium Used In Concrete, Mortar And Grout.

Never Step On Light Transmitting Panels (LTP's) Or Unattended Roof Panels

Panels May Collapse
If Not Properly Secured

Roof Panels Must Be Completely Attached To The Purlins And To Panels On Either Side Before They Can Be A Safe Walking Surface. Light Transmitting Panels LTP's) Translucent Panels Can Never Be Considered As A Walking Surface.

Partially Attached Or Unattached Panels Should Never Be Walked On!

Do Not:

1. Step On Rib At Edge Of Panel.
2. Step Near Crease In Rib At Edge Of Panel.
3. Step Within 5 Feet Of Edge On Unsecured Panel.

A Single Roof Panel Must Never Be Used As A Work Platform. An OSHA Approved Runway Should Be Used For Work Platforms. (Consult OSHA Safety And Health Regulations For The Construction Industry). Safety First!

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:04:07 -06'00'

STATE OF ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

Page	R1
Date	Jul '17
Rev	07

Erection Guide

Revision	Date	Description	By	Ck'd

CECO Building Systems
Columbus, MS. (662) 243-6400
Mount Pleasant, IA. (319) 385-8001
Rocky Mount, NC. (252) 977-2131
www.cecobuildings.com

Customer: MAD DASH INC. DBA SOUTHERN STEEL ST, 2210 MAIN ST STE G, DAPHNE, AL 36526-4546 US, ATTN: CORNELIUS LINDA

Project Name & Location: MAD DASH INC. DBA SOUTHERN STEEL ST, 19121 YOUNG ST, FAIRHOPE, AL 36532-1619 US

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

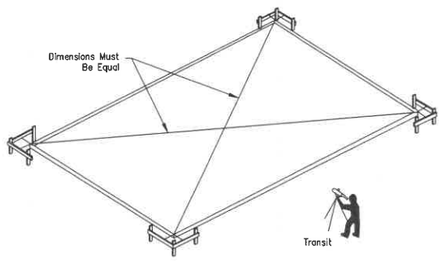
Scale:	NOT TO SCALE	
Drawn by:	ALN	2/3/23
Checked by:	ABE	2/7/23
Project Engineer:		
Job Number:	19-B-27589-1	
Sheet Number:	R1 of 9	

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

M.W. Custer, P.E.
Alabama P.E. PE21880

Building Anchorage

1. To Determine That The Foundation Is Square, Measure Diagonal Dimensions To Be Sure They Are Of Equal Length.
2. To Determine That The Foundation Is Level, Set Up A Transit Or Level And Use A Level Rod To Obtain The Elevation At All Columns.
3. Carefully Check The Location Of All Anchor Rods Against The Anchor Rod Setting Plan Furnished By The Manufacturer. All Dimensions Must Be Identical To Assure A Proper Start-up.



Pre-Erection Notes:

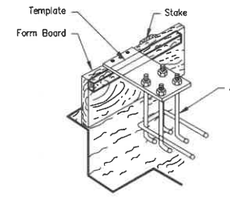
The Following Notes, Procedures And Suggested Recommendations Are Important Parts Of The Pre-Erection Process.

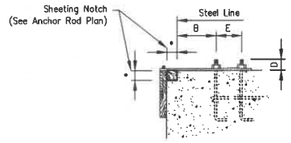
1.) Prior To The Time The Erection Crew Arrives, A Responsible Person Should Check The Job Site For Foundation Readiness, Square, And Accuracy And Anchor Rod Size And Location.

The Drawing Shown Below Indicates A Method Which May Be Used To Check The Foundation And Bolts For Square.

Measure Along Adjacent Sides Of Foundation Using A Pair Of Dimensions Shown. If The Diagonal Distance Between These Points Is As Noted, The Corner Is Square. Diagonal Measurements Between Opposite Anchor Rods Will Indicate If These Bolts Are Set Square.

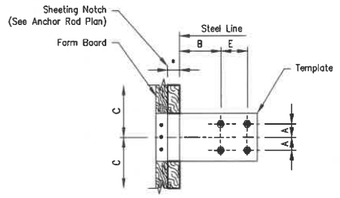
It Is Extremely Important That Anchor Rods Are Placed Accurately And In Accordance With The Anchor Rod Setting Plan. All Anchor Rods Should Be Held In Place With A Template Or Similar Means, So That They Will Remain Plumb And In Correct Location During The Placement Of The Concrete. A Final Check Should Be Made After Completion Of The Concrete Work And Prior To The Steel Installation. This Will Allow Necessary Corrections To Be Made Before Costly Installation Labor And Equipment Arrives.





Sheeting Notch Shown But May Not Be Required (See Anchor Rod Plan)

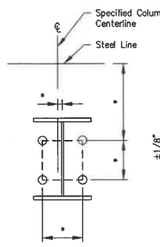
Projection Of Anchor Rods (D) Given On Anchor Rod Plan



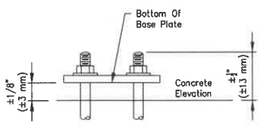
Dimensions A, B, And C Given On Anchor Rod Plan

AISC Code Of Standard Practice For Steel Building And Bridges Tolerances For Setting Anchor Rods

Anchor Rod Diameter, Inches (mm)	*Horizontal Variation, Inches (mm)
¾" and ⅞" (19 And 22 mm)	¼" (6 mm)
1", 1¼", 1½" (25, 31, 38 mm)	7/16" (10 mm)
1¾", 2", 2¼" (44, 50, 63 mm)	½" (13 mm)



ANCHOR ROD SETTING TOLERANCES



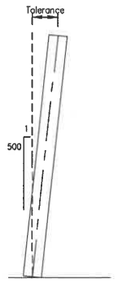
BASE PLATE AND ANCHOR ROD TOLERANCES

* Horizontal Variations Vary Depending On Anchor Rod Diameter. See Above

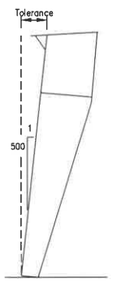
Erection Tolerances

ERECTION BRACING:
It Is The Responsibility Of The Erector To Determine, Furnish And Install All Temporary Supports Such As Temporary Guys, Beams, Falsework, Cribbing, Or Other Elements Required For The Erection Operation (In Accordance With Section 7.10.3 Of ANSI/AISC 303, Code Of Standard Practice For Steel Building And Bridges).

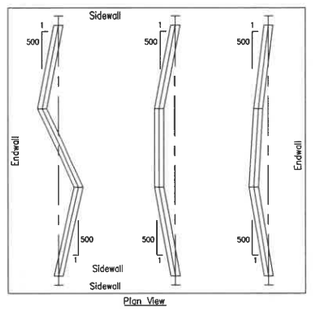
COLUMN ALIGNMENT TOLERANCES



Height	Tolerance (±) H/500
10'	3/4"
12'	7/8"
15'	1"
20'	1 1/2"
25'	1 7/8"
30'	2"
45'	3 3/8"
60'	4 7/8"

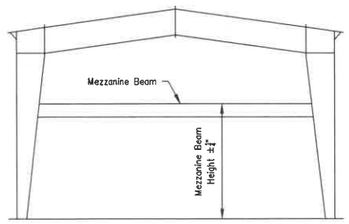


ALIGNMENT TOLERANCE FOR MEMBERS WITH FIELD SPLICES



Plan View

MEZZANINE BEAM HEIGHT TOLERANCE



General Erection Notes

1.) All Structural Framing Members, Purlins, Girts, Clips, Flange Braces, Bolts, Bracing Systems, Roof And Wall Panels, Etc. Must Be Installed As Shown On Erection Drawings.

2.) It Is Extremely Important, Especially During Construction, That Panels At The Eaves, Rakes And Ridges Be Kept Secure.

Panel Cautions And Notes

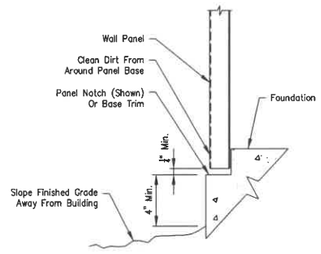
To Minimize Potential Of Corrosive Action At The Bottom Edge Of Wall Panels, The Contractor Must Assure That The Following Procedures Are Followed:

1.) The Concrete Foundation Should Be Cured For A Minimum Of Seven (7) Days Before Wall Panels Are Installed. (Uncured Concrete Is Highly Alkaline And Metal Panels Can Undergo Varying Degrees Of Corrosive Attack When In Direct Contact With The Concrete.) After The First Week Of The Curing Cycle, The Reaction Between Metallic Coatings On Steel And The Concrete Is Essentially Halted.

2.) Top Of Finish Grade At Building To Be A Minimum Of Four (4) Inches Below Bottom Of Panel.

3.) Finish Grade Is To Slope Away From Building To Ensure Proper Drainage.

4.) Upon Completion Of Finish Grading, All Dirt Is To Be Cleaned From Around Base Of Wall Panel Where It May Have Collected In Panel Notch Or On Base Trim.



Fastener Installation

Correct Fastener Installation Is One Of The Most Critical Steps When Installing Roof/Wall Panels. Drive The Fastener In Until It Is Tight And The Washer Is Firmly Seated. Do Not Overdrive Fasteners.
A Slight Extrusion Of Neoprene Around The Washer Is A Good Visual Tightness Check. Always Use The Proper Tool To Install Fasteners. A Fastener Driver (Screw Gun) With A RPM Of 1700-2000 Should Be Used For Self-Drilling Screws. A 500-600 RPM Fastener Driver Should Be Used For Self-Tapping Screws. Discard Worn Sockets, These Can Cause The Fastener To Wobble During Installation.

Note: Always Remove Metal Filings From Surface Of Panels At The End Of Each Work Period. Rusting Filings Can Destroy The Paint Finish And Void Any Warranty.

Correct Compression Of Sealing Washer | Too Tight Compression Of Sealing Washer | Too Loose Compression Of Sealing Washer

Tape And Tube Sealant

Proper Tape And Tube Sealant Application Is Critical To The Weather Tightness Of A Building. Tape Sealant Should Not Be Stretched When Installed. Apply Only To Clean, Dry Surfaces. Keep Only Enough Sealants On The Roof That Can Be Installed In A Day. During Warm Weather, Store Sealants In A Cool Dry Place. During Cold Weather (below 60°) Sealants Must Be Kept Warm (60°-90°) Until Application. After Tape Sealant Has Been Applied, Keep Protective Paper In Place Until Panel Is Ready To Be Installed.

Important Note

All Details, Recommendations And Suggestions Contained In This Erection Guide Of This Drawings Set Are For General Guidelines Only, And Not Meant To Be All-inclusive. Industry Accepted Installation Practices With Regard To All Areas Not Specifically Discussed In This Section Should Be Followed. Only Experienced, Knowledgeable Installers Familiar With Accepted Practices Should Be Used To Assure A Quality Project.

It Is Emphasized That The Manufacturer Is Only A Manufacturer Of Metal Building Components And Is Not Engaged In The Installation Of Its Products. Opinions Expressed By The Manufacturer About Installation Practices Noted In The Erection Guide Are Intended To Represent Only A Guide. Both The Quality And Safety Of Installation And The Ultimate Customer Satisfaction With The Completed Building Are Determined By The Experience, Expertise And Skills Of The Installation Crews, As Well As The Equipment Available For Handling The Materials. Actual Installation Operations, Techniques And Site Conditions Are Beyond The Manufacturers Control.

Revision	Date	Description	By	Ck'd

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2210 MAIN ST STE G
ATTN: CORNELIUS LINDA

☐ Preliminary (Not For Construction)
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Drawing Status:

Scale:	NOT TO SCALE
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Project Engineer:	
Job Number:	19-B-27589-1
Sheet Number:	R2 of 9

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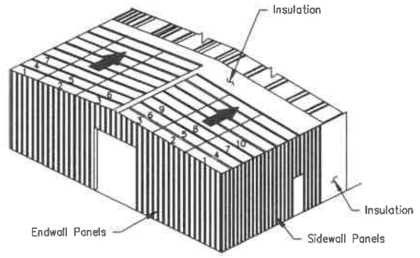
M.W. Custer, P.E.
Alabama P.E. PE21880

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:04:19 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

PBR Roof Panels

For PBR Roofs With Ridge Panels, It Is Recommended That Both Sides Of The Ridge Be Sheeted Simultaneously. This Will Keep The Insulation Covered For The Maximum Amount Of Time And The Panel Ribs Can Be Kept In Proper Alignment For The Ridge Panel. This Is Critical On The PBR Panels So That The Ridge Caps Can Be Properly Installed. Check For Proper Coverage As The Sheeting Progresses.



Install The First Run Of Roof Panels Across The Building From Eave To Eave Or Eave To Ridge. To Allow Proper Installation Of The Rake Trim, The Starting Location For The First Panel Must Be As Shown In The Rake Details Included With The Erection Drawings. When The First Run Is Properly Located And Aligned With The Correct Endlaps And Eave Overhangs, Fasten To Purlins. Roof Panels Should Be Installed So That The Sidelap Is In A Direction Away From Prevailing Wind. Refer To Appropriate Lap Details Included With The Erection Drawings.

Install Remaining Roof Insulation And Panels. To Avoid Accumulative Error Due To Panel Coverage Gain Or Loss, Properly Align Each Panel Before It Is Fastened. Occasional Checks Should Be Made To Ensure That Correct Panel Coverage Is Maintained. Special Attention Should Be Given To Fastener, Sealant and Closure Requirements. Refer To Details Included With The Erection Drawings.

At Finishing End Of Roof, The Last panels May Require Field Modification For Installation Of Rake Trim. Refer To Rake Details Included With The Erection Drawings. DO NOT BACK LAP THROUGH FASTENED ROOF PANELS.

NOTE: Roof Types And Installation Requirements Will Vary. Refer To The Appropriate Details For Specific Panel Used.

IMPORTANT: Loose Fasteners, Blind Rivets, Drill shavings, **Etc.**. Must Be Removed From The Roof To Guard Against Corrosion.

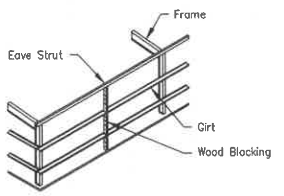
Wall Panels

Proper Horizontal And Vertical Alignment Of Supporting Structure (Girts Or Other Framing) Is The Responsibility Of The Installer. Failure To Align The Secondary members Properly Prior To Wall Installation Can Have A Direct Impact On The Final Appearance And Performance Of The Installed Wall System For Which The Metal Building Manufacturer Is Not Responsible.

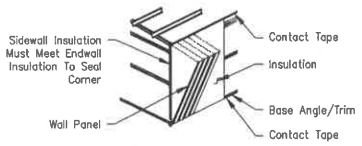
Before Installing Wall Panels, The Girts Must Be Aligned To A Level Position So That There Is No Visible Sag. This Should Be Done Directly Ahead Of Panel Installation.

Girt Leveling May Be Accomplished By Standing A Section Of Gable Angle Vertically Against The Outside Girt Flanges At Approximate Mid-bay Location. When Girts Are Level, Attach The Girt Flanges To The Angle With Vise Grip Pliers Or Temporary Screws. Wood Blocking Cut To Fit The Spaces May Also Be Used For Alignment.

Note:
Temporary Girt Blocking Is Not Recommended On Concealed Fastener Panels. The Removal Of The Blocks After Panel Installation Can Cause Oil Canning.



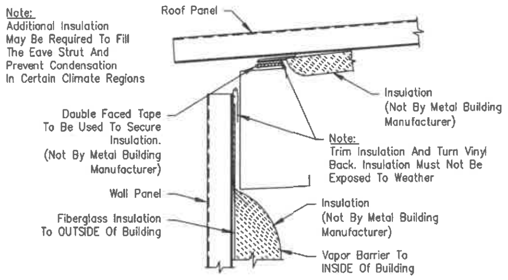
Note:
Wall Panel Type And Installation Details Will Vary. Refer To The Erection Drawings And Details For The Specific Panel Used For Your Building.



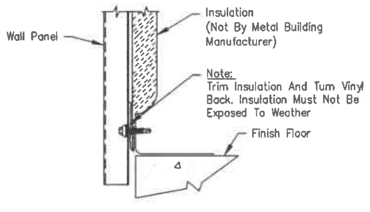
If Walls Are To Be Insulated With Blanket Insulation Over Girt Girt Flanges, Base And Eave, Place A Continuous Run Of Contact Tape Along The Eave Strut And Base Member.

Note:
At The Base, Cut Off The Insulation A Minimum Of $\frac{3}{4}$" Above The Bottom Of The Wall Panel. This Will Prevent The Insulation From Hanging Below The Wall Panel And Wicking Moisture.

Note:
Additional Insulation May Be Required To Fill The Eave Strut And Prevent Condensation In Certain Climate Regions



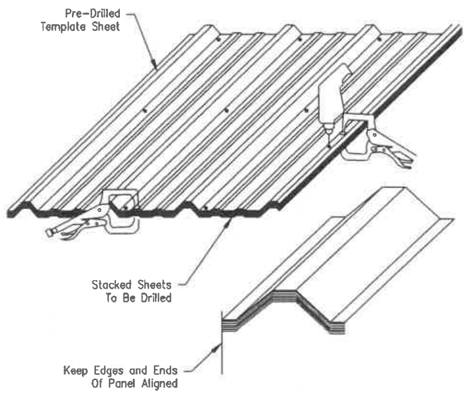
Eave Detail
(See Erection Drawings)



Base Detail
(See Erection Drawings)

Sidewall Panels Should Be Installed So That The Panel Sidelap Is In A Direction Away From The Prevailing Wind. Refer To Appropriate Lap Detail Included With Erection Drawings.)

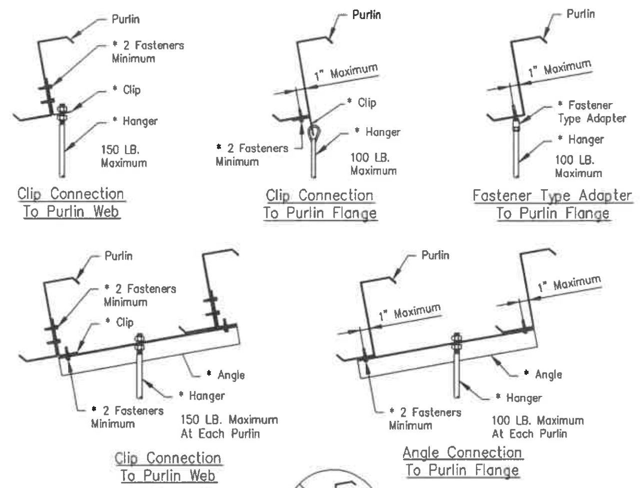
Note:
Check Periodically To Ensure That All Panels Are Aligned And Plumb.

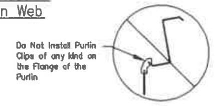


Screw Alignment Panel
(Through Fastened Panel Only)

Note:
After Drilling Panels, It Is Important To Clean Metal Filings Off All Panel Surfaces, Including Between Panels That Are Not Installed That Day, To Avoid Rust Stains.

Suggested Method Of Purlin Attachment For Building Accessories

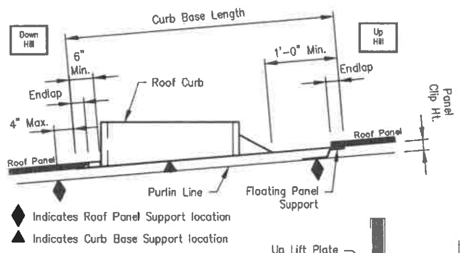


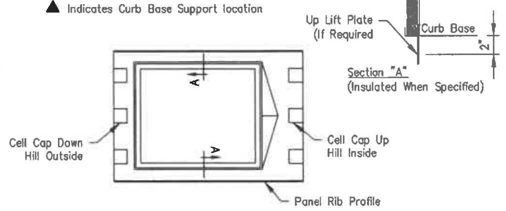


* Denotes Material Not Provided By Metal Building Manufacturer.

The Total Hanger Load Shall Not Exceed The Design Collateral Load For The Building. Example:
5'–0 (Purlin Spacing) X 5'–0 (Hanger Spacing) X 6 PSF (collateral Load) = 150 Lbs.
See Cover Sheet For Design Collateral Load For This Building.
Note: If The Building Is Designed For 0 PSF Collateral Load, Then Adding Any Suspended System (i.e. Duct Work, Piping, Lights, Ceilings, Etc.) Will Correspondingly Reduce The Design Live Load.

Roof Curbs When Not Supplied By Building Manufacturer



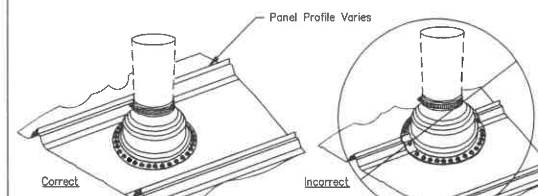


The Curb Details Shown Illustrate The Building Manufacturers Recommended Curb Style And Installation Method. It Is The Erector/Installer's Responsibility To Provide The Proper Curb Style And Install Them In Accordance With The Procedures Established By These Details. Failure By The Erector/Installer To Follow These Recommendations May Result In The Curbs Damaging The Roof System Or Excluded From Warranties.

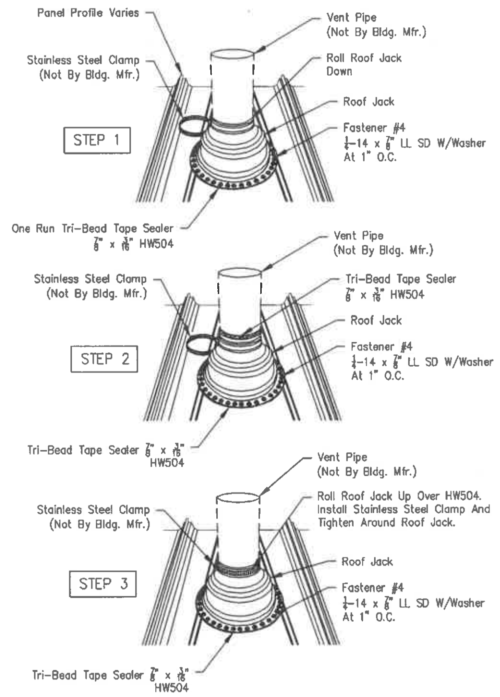
All Roof Curbs To Be:
1. .080 Aluminum Or 18 Ga. Stainless Steel (No Galvalume® Or Galvanized).
2. Panel Rib To Panel Rib (No Flat Skirt Or Lay-Over Curbs).
3. Installed With Down Hill End Over Panel And Up Hill End Under Panel Application For Water Flow At Panel Splice.
4. Up Lift Prevention For Clip Applied Roof Systems Are Required If:
   a. Wind Loads Exceed 110 MPH.
   b. Curb Base Crosses A Purlin.
5. Supported on (4) Sides By Primary Or Secondary Framing.
6. Maximum Single Curb Weight Recommended Is 1500 Lbs.

Roof Jack Installation when Not Supplied By Building Manufacturer

General Installation Notes
- Do Not Use Galvanized Roof Jacks, Lead Hats, Or Other Residential Grade Roof Jacks. These Roof Jacks Do Not Have 20 Year Service Life And In Case Of Lead Hats Will Cause Galvanic Corrosion Of The Roof Panel.
- Use EPDM Rubber Roof Jacks With An Integral Aluminum Band Bonded Into The Perimeter Of The Base. EPDM Roof Jacks Have A Temperature Range From –65°F To 212°F. Use Silicone Roof Jacks For High Temperatures. Silicone Roof Jacks Have A Temperature Range Of –100°F To 437°F.
- Retrofit Roof Jacks Are Available For Applications In Which The Top Of The Pipe Is Inaccessible, Eliminating The Possibility Of Sliding The Roof Jack Over The Top Of The Pipe.
- Do Not Use Tube Sealant To Seal The Roof Jack To The Roof Panels. Use Roll Tape Sealer Between The Roof Jack And The Roof Panel And Attach The Roof Jack To The Roof Panel With Fastener #4 $\frac{1}{4}$ – 14 X $\frac{7}{8}$" LL SD W/washer At 1" O.C. Around The Base Of The Roof Jack. See Table Below For Quantities.
- Trim The Top Of The Roof Jack To Fit Over The Pipe, Roll Down The Roof Jack Over The Pipe And Apply Tape Sealer For The Perimeter Of The Roof Jack Base Between The Roof Jack And The Roof Panel. Apply Tape Sealer Around The Pipe And Install A Stainless Steel Clamp (Not By Bldg. Mfr.) Over The Top Of The Roof Jack And Firmly Tighten To Form A Secure Compression Seal.
- If The Pipe Diameter Is So Large To Block The Flow Of Water Down The Roof Panel, A Flat Base Roof Curb Must Be Installed Into The Roof And The Roof Jack Will Be Sealed To The Curb. A Two Piece Curb May Be Required When The Top Of The Pipe Is Inaccessible.
- In Northern Climates, The Pipe Penetration Should Be Protected From Moving Ice Or Snow With A Snow Retention System Immediately Up Slope From The Pipe.



Install Pipe In Center To Allow Base Of Roof Jack To Lay Flat on Panel. Cannot Encompass More Than 75% Of Panel.



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Customer:
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ATTN: CORNELIUS LINDA

Project Name & Location:
MAD DASH INC. DBA SOUTHERN STEEL ST
19121 YOUNG ST
FAIRHOPE, AL 36532-1619 US

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale: NOT TO SCALE
Drawn by: ALN 2/3/23
Checked by: ABE 2/7/23
Project Engineer:
Job Number: 19-B-27589-1
Sheet Number: R3 of 9

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M.W. Custer, P.E.
Alabama P.E. PE21880

MICHAEL W CUSTER
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Date: 2023.02.08 12:04:33 -06'00'

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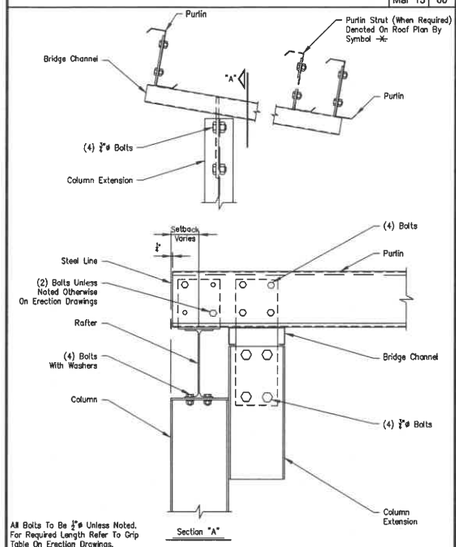
Bearing Frame Endwall - Inside Column Extension to Bridge Channel Between Purlins - Flush

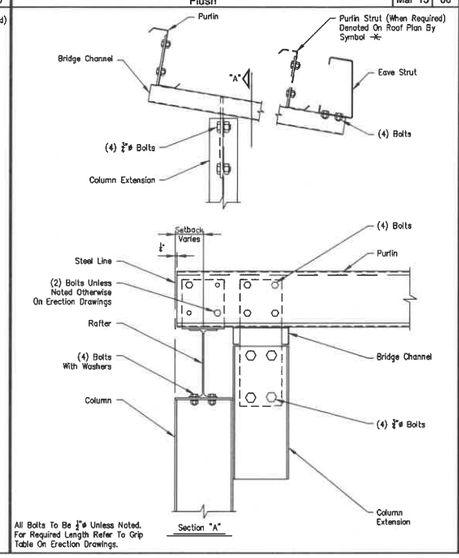
Page EF07112 | Date Mar '15 | Rev 00



All Bolts To Be ¾"ø Unless Noted. For Required Length Refer To Grip Table On Erection Drawings.

Bearing Frame Endwall - Inside Column Extension to Bridge Channel Between Low Side Eave Strut and Purlin (Roof Slopes 4½:12 or Less) Flush

Page EF07117 | Date Mar '15 | Rev 00



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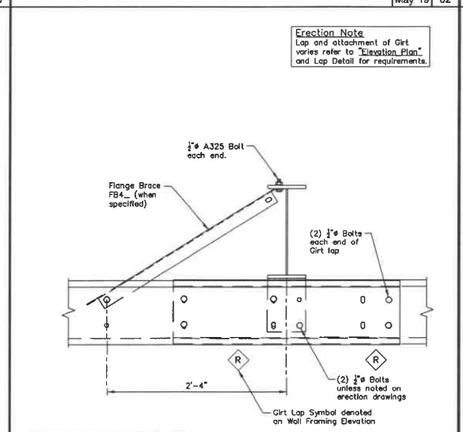
By-Pass Girt to Column - Standard Connection - Flange Brace FB4_

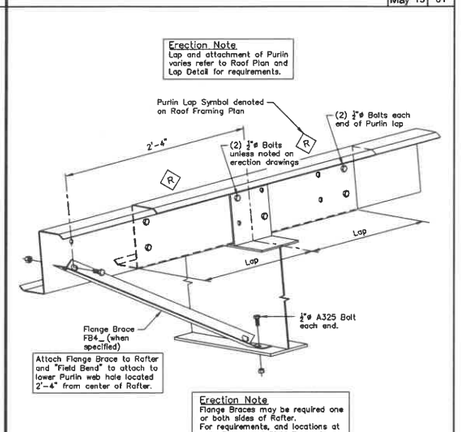
Page CF01124 | Date May '19 | Rev 02



Erection Note
Flange Braces may be required one or both sides of Column.
For requirements, and locations at Main Frame and Rigid Frame in Endwall refer to "Cross Section".
For requirements, and locations at Bearing Frame Endwall refer to "Elevation Plan".

Purlin to Rafter - Standard Connection - Flange Brace FB4_

Page CF02036 | Date May '19 | Rev 01

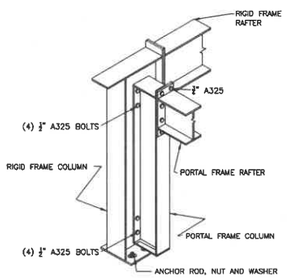


Attach Flange Brace to Rafter and "Field Bend" to attach to lower Purlin web hole located 2'-4" from center of Rafter.

Erection Note
Flange Braces may be required one or both sides of Rafter.
For requirements, and locations at Main Frame and Rigid Frame in Endwall refer to "Cross Section".
For requirements, and locations at Bearing Frame Endwall refer to "Roof Plan".

PRIMARY FRAME DETAIL - WEB BOLTED NESTED PORTAL FRAME ONE HALF STIFFENER

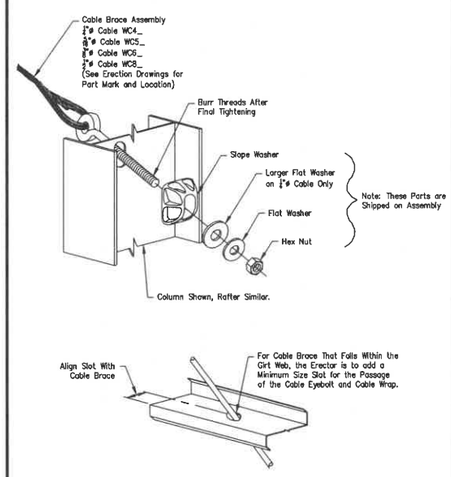
Page PF04016-X | Date Mar '22 | Rev 02



NOTE: FRAME SHAPES MAY VARY.
SEE FRAMING PLAN FOR PORTAL FRAME LOCATIONS

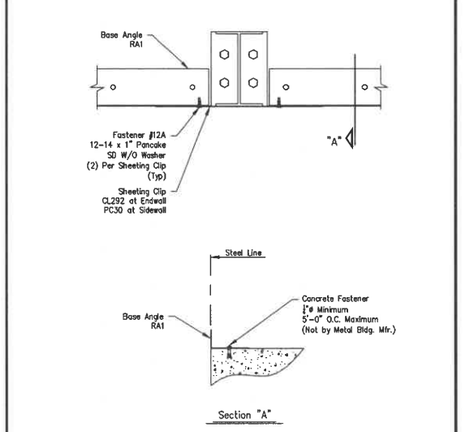
Cable Brace Attachment at Web

Page PF04111 | Date May '17 | Rev 02



Column Base Angle Detail With Flush Column

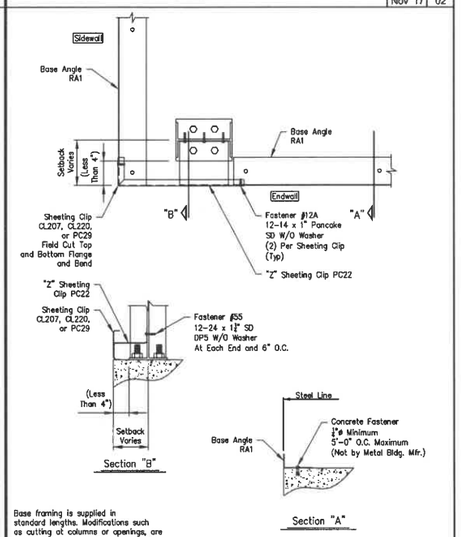
Page BF01005 | Date Apr '16 | Rev 00



Base framing is supplied in standard lengths. Modifications such as cutting at columns or openings, are to be handled in the field.

Corner Base Angle Detail With By-Pass Sidewall Column Setback Less Than 4"

Page BF01013 | Date Nov '17 | Rev 02



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MICHAEL W CUSTER

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Date: 2023.02.08 12:04:46 -06'00'

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Project Engineer:	
Job Number:	19-B-27589-1
Sheet Number:	R4 of 9

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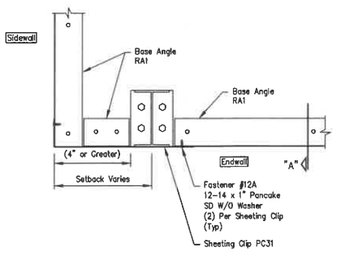
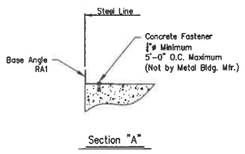
M.W. Custer, P.E.
Alabama P.E. PE21880

Corner Base Angle Detail With Flush Endwall Column
Sidewall Base Angle By-Passes Column Base Plate

Page BF01021 | Date Apr '16 | Rev 00



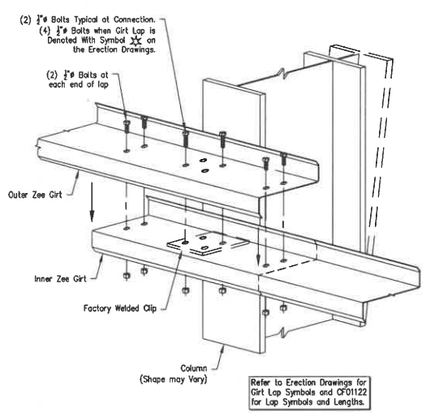


Section "A"

Base framing is supplied in standard lengths. Modifications such as cutting at columns or openings, are to be handled in the field.

Girt Connection
At Bypass Column

Page CF01120 | Date Jun '16 | Rev 02

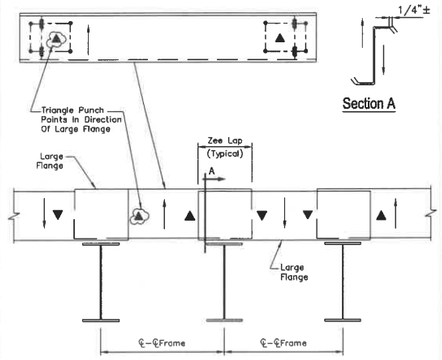


Refer to Erection Drawings for Girt Lap Symbols and CF01122 for Lap Symbols and Lengths.

All Bolts to be ¾"ø Unless Noted. For Required Length Refer to Grip Table on Erection Drawings.

Bypass Zee Installation

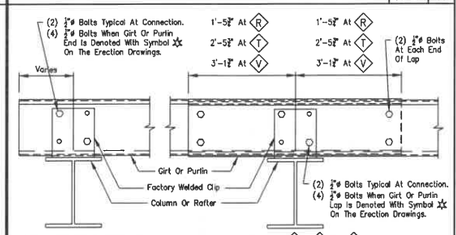
Page CF01121 | Date Apr '19 | Rev 01



The large leg of the Zee must be alternated from top to bottom in order to nest the member correctly. A triangle has been added to the end of the Zee near the connection holes, that will point to the large leg of the member.

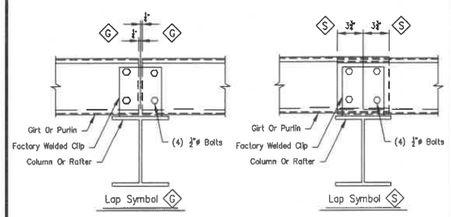
Girt And Purlin Connection
At Bypass Column And Rafter

Page CF01122 | Date Jul '17 | Rev 03



Connection At End Frame

Lap Symbol R, T & V



Lap Symbol G | Lap Symbol S

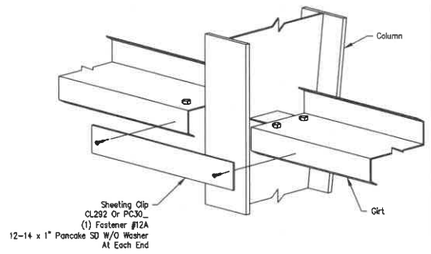
ZEE SECTION LAP TABLE			
SYMBOL	LAP LENGTH	SYMBOL	LAP LENGTH
R	-0'-0½"	S	2'-5½"
T	0'-3½"		3'-1½"
V	1'-5½"		

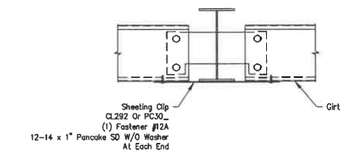
Refer to Erection Drawings for Lap Symbol at Girt and Purlin Connection.

All Bolts to be ¾"ø Unless Noted. For Required Length Refer to Grip Table on Erection Drawings.

Girts At Column
Flush Sidewall Or Endwall Girts

Page CF01123 | Date Apr '19 | Rev 02

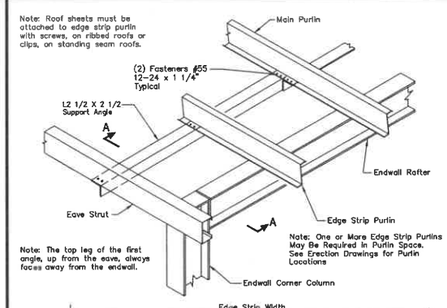


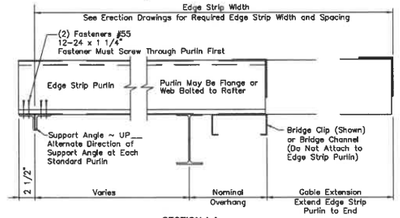


Notes:
Zee Girt Shown
Cee Girt Similar
All Bolts To Be ¾"ø Unless Noted
For Required Length Refer To Grip Table On Erection Drawings.

Rake Edge Strip Purlins - Partial Bay Length Purlin Details

Page CF02005 | Date May '19 | Rev 02

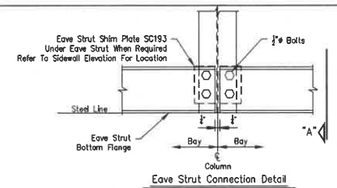




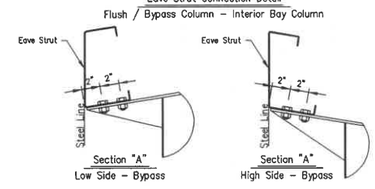
SECTION A-A

Eave Strut Connection
Flush / Bypass Column - Interior Bay Column - Roof Slope 4½:12 Or Less

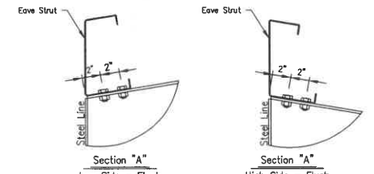
Page CF02030 | Date Sep '17 | Rev 02



Eave Strut Connection Detail
Flush / Bypass Column - Interior Bay Column



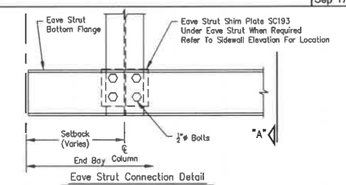
Section "A" Low Side - Bypass | Section "A" High Side - Bypass



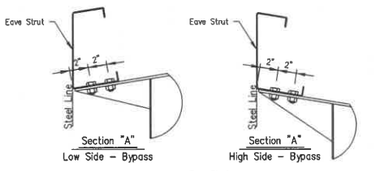
Section "A" Low Side - Flush | Section "A" High Side - Flush

Eave Strut Connection
Flush / Bypass Column - End Bay Column - Roof Slope 4½:12 Or Less

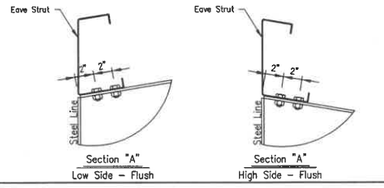
Page CF02033 | Date Sep '17 | Rev 04



Eave Strut Connection Detail
Flush / Bypass Column - End Bay Column



Section "A" Low Side - Bypass | Section "A" High Side - Bypass



Section "A" Low Side - Flush | Section "A" High Side - Flush

MICHAEL W CUSTER

Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:04:59 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

Revision	Date	Description	By	Ck'd

CECO Building Systems

Columbus, MS. (662) 243-8400
Mount Pleasant, IA. (319) 385-8001
Rocky Mount, NC. (252) 977-2151
www.cecobuildings.com

Project Name & Location:
MAD DASH INC. DBA SOUTHERN STEEL - LONG ST.
1921 LONG ST.
FAIRHOPE, AL 36532-1619 US

Customer:
MAD DASH INC. DBA SOUTHERN STEEL - LONG ST
2210 MAIN ST STE G
FAIRHOPE, ALABAMA 36532-2546 US

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

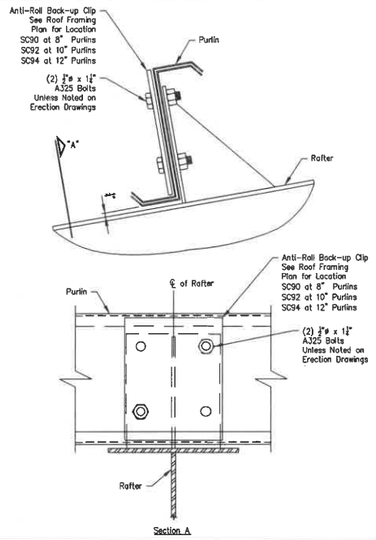
Scale:	NOT TO SCALE	
Drawn by:	ALN	2/3/23
Checked by:	ABE	2/7/23
Project Engineer:		
Job Number:	19-B-27589-1	
Sheet Number:	R5 of 9	

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

M.W. Custer, P.E.
Alabama P.E. PE21880

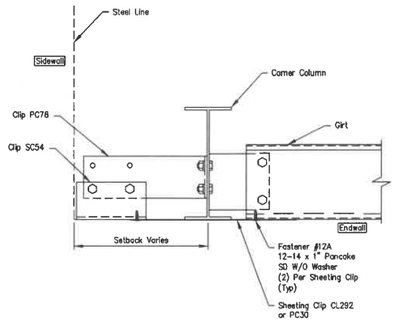
Purlin with Anti-Roll Back-up Clip at Rafter

Page CF02035 | Date Dec '14 | Rev 04



Endwall Girts and Non-Aligned Girts or Open Sidewall at Rotated Flush Corner Column

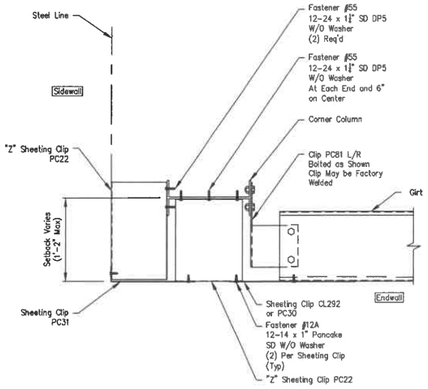
Page EF03205 | Date Feb '17 | Rev 02



Note:
Zee Girt Shown
Cee Girt Similar
All Bolts To Be ¾" Unless Noted.
For Required Length Refer To Grip
Table On Erection Drawings.

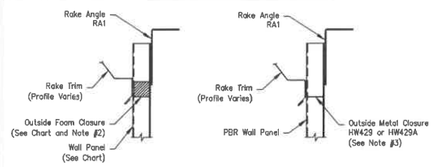
Bypass Sidewall and Endwall Girts - No Sidewall Girt - Variable Frame Setback - Endwall Girt Attaches to Inner Column Flange

Page EF03517 | Date Feb '17 | Rev 00

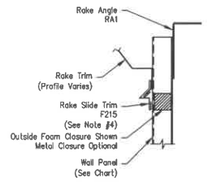


Single Skin Wall Panel Outside Closure Requirements at Rake

Page GD06003 | Date Feb '19 | Rev 02



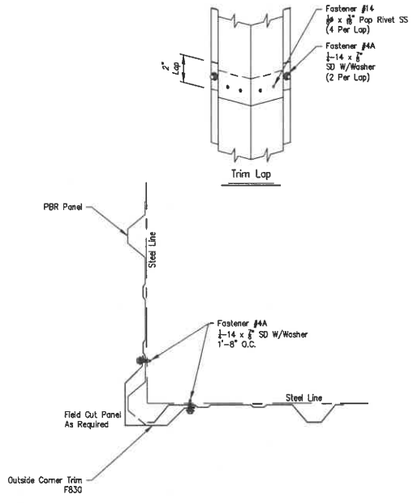
Notes:
1. Outside Panel Closures are Required at all Sheeted Endwalls.
2. PBR Wall Panel Outside Foam Closures HW456 Required for Roof Slope 4:12 or Less and HW422 for Roof Slope Greater Than 4:12. Field Form/Notch HW422 to Panel Profile.
3. PBR Wall Panel Outside Metal Closure HW429 for Roof Slope 0 :12 Thru 1½ :12 and HW429A for Roof Slope Greater Than 3½ :12 Thru 4½ :12.
4. Rake Slide Trim Required for All Standing Seam Roofs and at Screw Down Roof Runs Greater Than 100'-0"
5. Foam Closures are Required when Job Requires Air Infiltration or Sealed Wall Requirements, See GD1600Z.



Wall Panel	Foam Closure
PBR	HW456/HW422
AVP	HW465
PBU	HW460
VistaShadow	HW465
NuWall	HW424
PBC	HW462
PBD	HW463
ShadowRib	HW412
Designer Series (Fluted Only)	HW4037
RBR (Reverse Rolled PBR)	HW455
RBU (Reverse Rolled PBU)	HW459
7.2	HW461

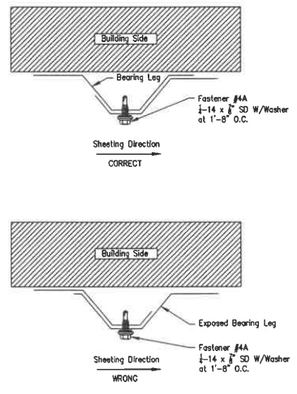
PBR Wall Panel Outside Corner - On Module

Page PW03001 | Date Apr '19 | Rev 08



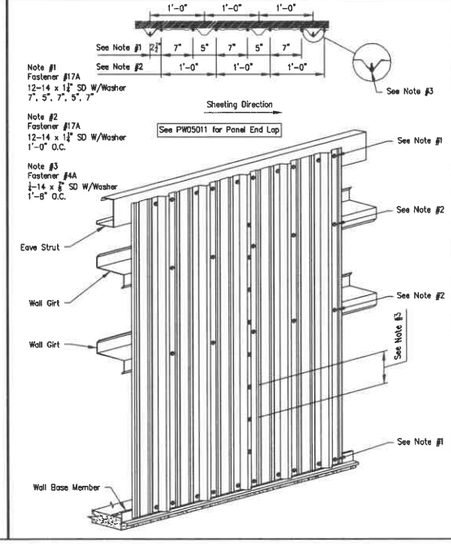
PBR Wall Panel Panel Side Lap

Page PW05002 | Date Mar '19 | Rev 03



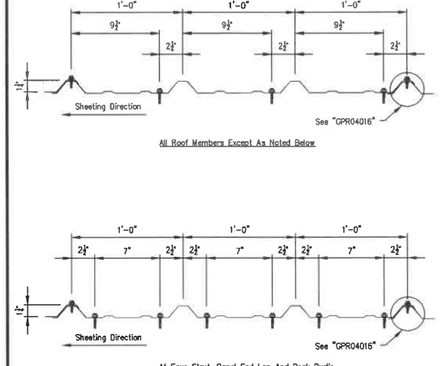
PBR Wall Panel Fastener Location

Page PW05003 | Date Aug '15 | Rev 04



PBR Roof Panel Fastener And Tape Sealant Location

Page GPR00011 | Date Apr '19 | Rev 01



Note:
Screw patterns shown satisfy U.L. 90 requirements for roof.

Revision	Date	Description	By	Ck'd

CECO Building Systems
Columbia, MS (662) 243-5400
Mount Pleasant, IA (319) 385-8001
Rocky Mount, NC (252) 977-2131
www.cecobuildings.com

Project Name & Location:
MAD DASH INC. DBA SOUTHERN STEEL ST
19121 YOUNG ST
FAIRHOPE, AL 36532-1619 US

Customer:
MAD DASH INC. DBA SOUTHERN STEEL ST
2210 MAIN ST STE G
LAKE ALFRED, FL 33850 US
ATTN: CORNELIUS LINDA

Drawing Status:
☐ Preliminary
☐ Not For Construction
☐ For Approval
☐ Not For Construction
☒ For Construction Permit
☐ For Erector Installation

Scale:	NOT TO SCALE
Drawn by:	ALN 2/3/23
Checked by:	ABE 2/7/23
Project Engineer:	
Job Number:	19-B-27589-1
Sheet Number:	R6 of 9

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M.W. Custer, P.E.
Alabama P.E. PE21880

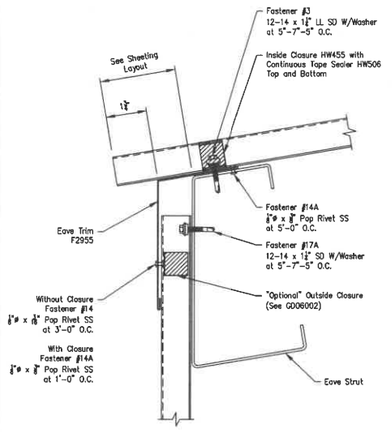
MICHAEL W CUSTER

Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:05:33 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

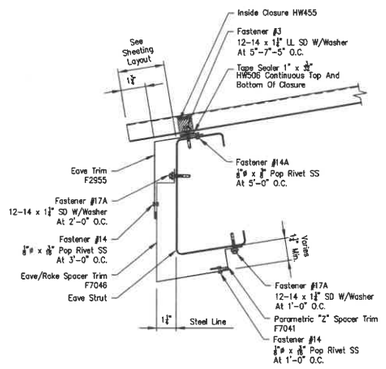
PBR Panel
Flat Eave Trim - Sheeted Wall

Page GPR03004 | Date Feb '17 | Rev 05



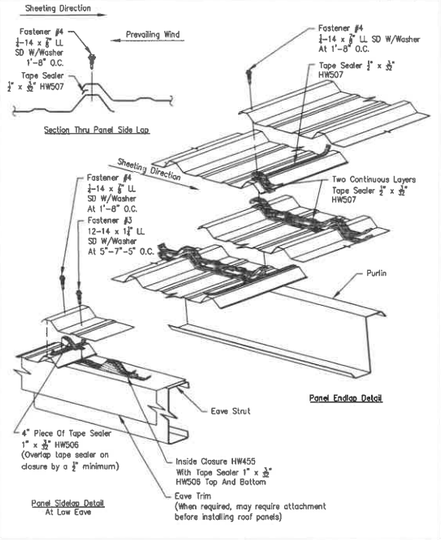
PBR Panel - Flat Eave Trim
Partially Or Fully Open Wall

Page GPR03007 | Date Dec '21 | Rev 00



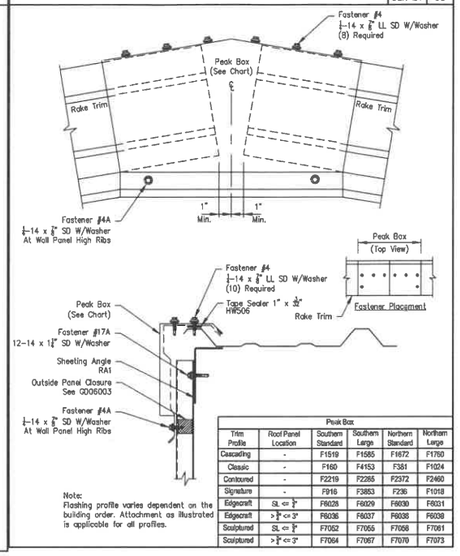
PBR Roof Panel
Side Lap And End Lap Details

Page GPR04016 | Date Apr '19 | Rev 04



PBR Roof Panel
Peak Box At Fixed Ridge

Page GPR05001 | Date Jun '21 | Rev 08



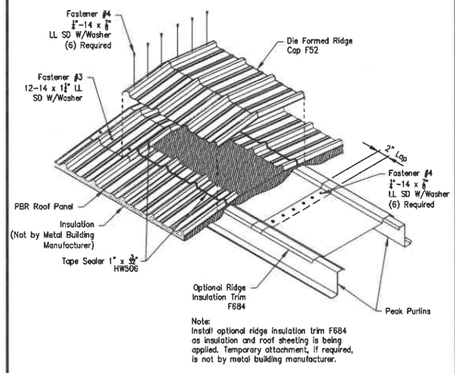
Peak Box

Trim Profile	Roof Panel Location	Southern Standard	Southern Large	Northern Standard	Northern Large
Cascading	-	F1519	F1585	F1672	F1760
Classic	-	F160	F4153	F381	F1024
Contoured	-	F2219	F2285	F2372	F2460
Signature	-	F916	F3853	F236	F1018
Edgecraft	SL <= 2"	F6028	F6029	F6030	F6031
Edgecraft	> 2" <= 3"	F6036	F6037	F6038	F6039
Sculptured	SL <= 2"	F7052	F7055	F7058	F7061
Sculptured	> 2" <= 3"	F7064	F7067	F7070	F7073

Note:
Flashing profile varies dependent on the building order. Attachment as illustrated is applicable for all profiles.

PBR Roof Panel
Fixed Ridge Detail

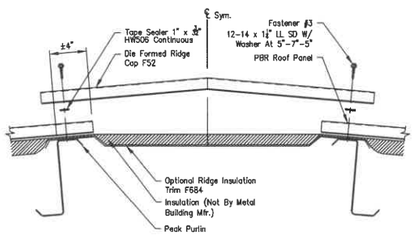
Page GPR06003 | Date Apr '19 | Rev 04



Note:
Install optional ridge insulation trim F684 as insulation and roof sheeting is being applied. Temporary attachment, if required, is not by metal building manufacturer.

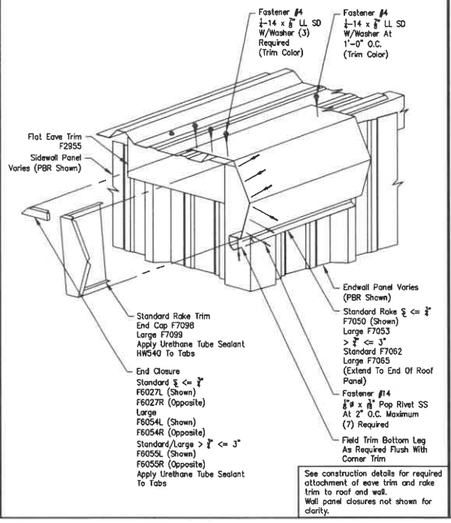
PBR Roof Panel
Section Thru Fixed Ridge

Page GPR06004 | Date Apr '19 | Rev 04



PBR Roof - Southern Standard And Southern Large Sculptured
Low Eave Rake Corner With Flat Eave Trim - ¾" Thru 1½" Wall Panel

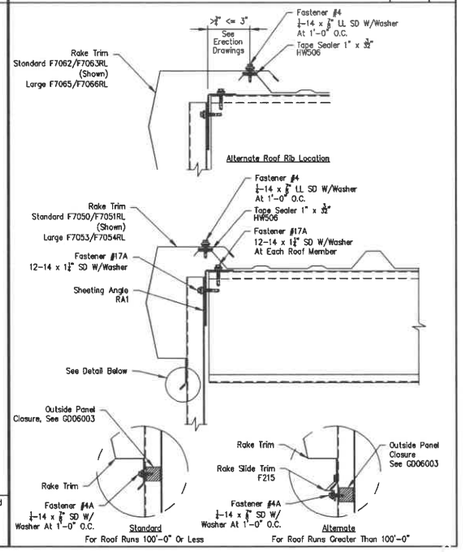
Page SPR04006 | Date Jun '21 | Rev 00



See construction details for required attachment of eave trim and rake trim to roof and wall. Wall panel closures not shown for clarity.

PBR Roof - Southern Standard And Southern Large
Sculptured Rake Trim - Sheeted Wall

Page SPR05002 | Date Jun '21 | Rev 00



CECO Building Systems

Columbus, MS (662) 245-8400
Mount Pleasant, IA (319) 385-6001
Rocky Mount, NC (252) 977-2101
www.cecobuildings.com

Revision	Date	By	Ck'd

Description

Project Name & Location:
MAD DASH INC. DBA SOUTHERN STEEL, LLC
1912 I YOUNG ST
FAIRHOPE, AL 36532-1619 US

Customer:
MAD DASH INC. DBA SOUTHERN STEEL, LLC
2210 MAIN ST, STE G
CONYERS, GA 30094-4546 US
ATTN: CORNELIUS LINDA

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale:	NOT TO SCALE	
Drawn by:	ALN	2/3/23
Checked by:	ABE	2/7/23
Project Engineer:		
Job Number:	19-B-27589-1	
Sheet Number:	R7 of 9	

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M.W. Custer, P.E.
Alabama P.E. PE21880

MICHAEL W CUSTER

Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:05:47 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

PBR Roof Panel - Perimeter Trim Reference
Trim Fasteners And Sealants

Page SPR00008 | Date May '22 | Rev 01

EAVE BOX TRIM F7040 20' Tube Sealant (8) Fastener #14 Trim Color EAVE BOX END CAP F7198RL	EAVE BOX TRIM LAP 2" LAP 22' Tube Sealant (12) Fastener #14 Trim Color	Closure Vented Ridge HW4040 (3) Fastener #3 12-14 x 1" LL SD W/Washer (2) Fastener #4 1/4-14 x ¾" LL SD W/Washer Roof Color 2'-4" Tape Sealer	Metal Closure HW4041 (3) Fastener #3 12-14 x 1" LL SD W/Washer (2) Fastener #4 1/4-14 x ¾" LL SD W/Washer Roof Color 2'-4" Tape Sealer	Support Angle Longitudinal Ridge SMRS 20'-0" Long
Expansion Ridge Trim F675 Fastener #4 1/4-14 x ¾" LL SD W/Washer 2 runs at 8" O.C. (16) Fastener #4 1/4-14 x ¾" LL SD W/ Washer Per Lap Trim Color 5'-6" Tube Sealant	Ridge Closure F679 2'-6" Tube Sealant (12) Fastener #4 1/4-14 x ¾" LL SD W/Washer Trim Color	Flat Eave Trim F2955 9" Tube Sealant (2) #14 per 2' Lap #14 3'-6" O.C W/O Closure #14A 1'-0" O.C. W/ Closure Trim Color	Inside Closure HW455	Outside Closure HW456
Parapet Rake Trim F852 2'-3" Tube Sealant (3) Fastener #4 ¾-14 x ¾" LL SD W/Washer Trim Color	Rake Side Trim F215 Fastener #4A 1/4-14 x ¾" SD W/O Washer 1'-0" O.C. Trim Color	Ridge Cap F52 10'-1" Tape Sealer (12) Fastener #3 12-14 x 1" LL SD W/Washer (6) Fastener #4 1/4-14 x ¾" LL SD W/Washer Panel Color	Parapet High Side F326 2'-8" Tape Sealant (2) Fastener #14 & (5) Fastener #4 1/4-14 x ¾" LL SD W/Washer Per Lap Fastener #4 1/4-14 x ¾" LL SD W/Washer 1'-0" O.C. Trim Color	

Fasteners

Page G000004 | Date Jul '17 | Rev 05

Fastener #14
1/8" x 3/16" Pop Rivet Stainless Steel

Fastener #14A
1/8" x 3/8" Pop Rivet Stainless Steel

Fastener #24
8 x 5/8" Ribbed Driller

Fastener #35
#14 x 1 1/8" O.D. Bonded Washer

Fastener #43L
L.T.P. Member Screw (Long Life) 1/4"-14 x 1 1/4" 5/16" Hex Washer Head W/ 1 1/8" O.D. Washer

Fastener #44L
L.T.P. Stitch Screw (Long Life) 1/4"-14 x 7/8" 5/16" Hex Washer Head W/ 1 1/8" O.D. Washer

Fastener #226
3/16" x 9/16" Closed End Rivet

Fastener #228
10 x 1/2" Grommet Washer

Fastener #271
8-18 x 1/2" Trim Screw

Fastener HW399
#6 x 1" Rubber Grommet 1/4" Hex Head w/ Washer

Note: Refer to bill of materials for specific job requirements.

Tape Sealer And Tube Sealant

Page G000005 | Date Apr '19 | Rev 05

TRI-BEAD TAPE SEALER HW504
1/8" X 3/8" X 25'-0"

TRIPLE BEAD TAPE SEALER HW502
1/8" X 2½" X 20'-0"

URETHANE TUBE SEALANT
HW540 (White)
HW541 (Gray)
HW542 (Bronze)
Note: 25'-0" per Tube at 1/4" Bead

DEKSTRIP 7" WIDE = HW5227
DEKSTRIP 9" WIDE = HW5228
DEKSTRIP 12" WIDE = HW5229
DEKSTRIP 18" WIDE = HW5226
COLOR = Gray
SCREWS 2" O.C. MAX. PERIMETER
TAPE SEALER BOTH SIDES
URETHANE TUBE SEALANT HW540 EACH END
TERMINATION STRIP HW5305 EACH END (1" Wide x 4'-0" Long Alum.)

FLAT TAPE SEALER HW507
1/32" X ½" X 50'-0"

FLAT TAPE SEALER HW506
1/32" X 1" X 45'-0"

TAPE SEALER MINOR RIB HW512
7/32" X 1⅜" X 4"

2" WIDE X 24 GA. STRAPPING
- FL470 – 25'-0" Roll Galvalume Plus Only
- FL471 – 100'-0" Roll Galvalume Plus Only
- FL569 – 500'-0" Roll Galvalume Plus or White Wash Coat

TAPE SEALER – SWAGED HW515
1/8" X 2½" X 6"
6"
BattenLok HS SuperLok

NON-SKINNING BUTYL TUBE (VAPOR SEALANT)
IMP7100 (WHITE)
Note: 12'-6" per Tube at 3/8" Bead

FLEXIBLE MEMBRANE (EPDM)
18" or 24"
- HW520 – 16" x 50'-0" Roll
- HW521 – 24" x 50'-0" Roll

NOTE: Refer to bill of materials for specific job requirements

PBR, PBU, AVP, Vistashadow, RBR, RBU
Panel Fasteners

Page G000006 | Date May '19 | Rev 09

Wall Fasteners	Roof Fasteners Long Life (Optional at Wall)
Member Screw Fastener #17A 12-14 x 1½" 5/16" Hex Washer Head w/washer	Member Screw Fastener #3 12-14 x 1¼" 5/16" Hex Washer Head w/washer
Member Screw Optional Fastener #17B 12-14 x 1½" 5/16" Hex Washer Head w/washer	Member Screw Optional Fastener #3A 12-14 x 1½" 5/16" Hex Washer Head w/washer
Member Screw Optional Fastener #28 12-14 x 2" 5/16" Hex Washer Head w/washer	Member Screw Optional Fastener #58 12-14 x 2" 5/16" Hex Washer Head w/washer
Stitch Screw Fastener #4A ¼-14 x ¾" 5/16" Hex Washer Head w/washer	Stitch Screw Fastener #4 ¼-14 x ¾" 5/16" Hex Washer Head w/washer

NOTE: Refer to bill of materials for specific job requirements

Various Fasteners

Page G000009 | Date Nov '16 | Rev 11

Fastener #17
12-14 x 1" SD W/Washer 5/16" Hex Head

Fastener #38
¼-14 x ⅞" SD W/O Washer 5/16" Hex Head

Fastener #12A
12-14 x 1" Pancake SD W/O Washer

Fastener #55
12-24 x 1½" SD DP5 W/O Washer 5/16" Hex Head

Fastener #70
12-24 x 1⅜" SD DP5 W/O Washer 5/16" Hex Head

Fastener #142
¼-14 x 1½" SD W/O Washer 5/16" Hex Head

Fastener #76
12-14 x 2" SD W/O Washer 5/16" Hex Head

Fastener #61
12-14 x 1⅜" SD W/O Washer 5/16" Hex Head

Fastener #18
¼-14 x 1½" SD W/O Washer 5/16" Hex Head

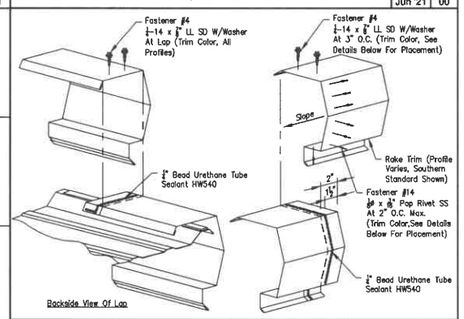
Fastener #16
12-24 x 1½" Pancake SD DP5 W/O Washer

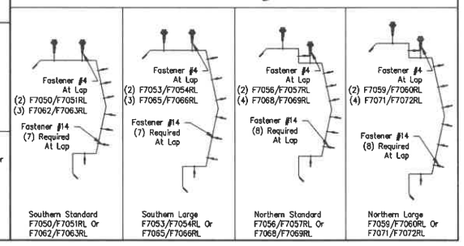
NOTE: Refer to Bill of Materials for Specific job Requirements

Fastener #46
¼-14 x ⅞" LL ST Type B W/Washer 5/16" Hex Head

PBR Roof - Sculptured Rake Trim
End Lap Installation

Page SPR05010 | Date Jun '21 | Rev 00





CECO Building Systems
Columbus, MS (662) 243-8400
Mount Pleasant, NC (210) 385-8001
Rocky Mount, NC (252) 977-2131
www.cecobuildings.com

Revision	Date	By	Ck'd

Description

Project Name & Location:
MAD DASH INC. DBA SOUTHERN STEEL ST
19121 YOUNG ST
FAIRHOPE, AL 36532-1619 US

Customer:
MAD DASH INC. DBA SOUTHERN STEEL ST
2210 MAIN ST STE G
DUNN, NC 28334-4546 US
ATTN: CORNELIUS LINDA

Drawing Status:
☐ Preliminary (Not For Construction)
☐ Approved (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

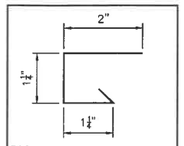
Scale:	NOT TO SCALE
Drawn by:	ALN 2/3/23
Checked by:	ABE 2/7/23
Project Engineer:	
Job Number:	19-B-27589-1
Sheet Number:	R8 of 9

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only.The undersigned engineer is not the overall engineer of record for this project.

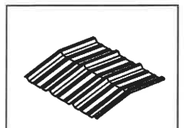
M.W. Custer, P.E.
Alabama P.E. PE21880

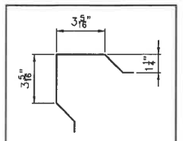
MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:06:00 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

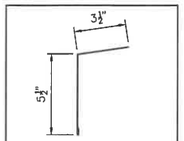


F22
Optional Jamb Trim


F52
Ridge Cap



F830
Outside Corner Trim



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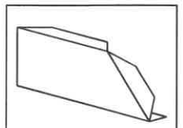
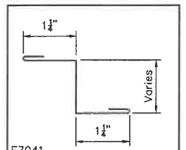
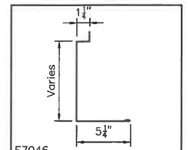
Flat Eave Trim


F6027RL
Rake Trim End Closure



F7041
Spacer Trim



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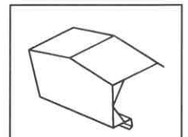
Spacer Trim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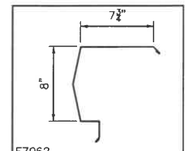
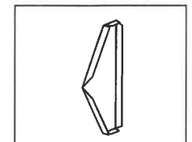
Peak Box



F7062
Rake Trim


F7098
Rake Trim End Cap

Revision	Date	Description	By	Ck'd

Columbus, MS. (662) 243-8400
Mount Pleasant, IA. (319) 385-8001
Rocky Mount, NC. (252) 977-2131
www.cecobuildings.com

CECO Building Systems

Project Name & Location:
MAD DASH INC. DBA SOUTHERN
STEEL - YOUNG ST
1971 YOUNG ST
FAIRHOPE, AL 36532-1619 US

Customer:
MAD DASH INC. DBA SOUTHERN STEEL ST
2210 MAIN ST STE G
KATHLEEN, GEORGIA 31047 US
ATTN: CORNELIUS LINDA

Drawing Status:
☐ Preliminary (Not For Construction)
☒ For Construction Permit
☐ For Approval (Not For Construction)
☐ For Erector Installation

Scale:	NOT TO SCALE	
Drawn by:	ALN	2/3/23
Checked by:	ABE	2/7/23
Project Engineer:		
Job Number:	19-B-27589-1	
Sheet Number:	R9 of 9	

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

M.W. Custer, P.E.
Alabama P.E. PE21880

MICHAEL W CUSTER

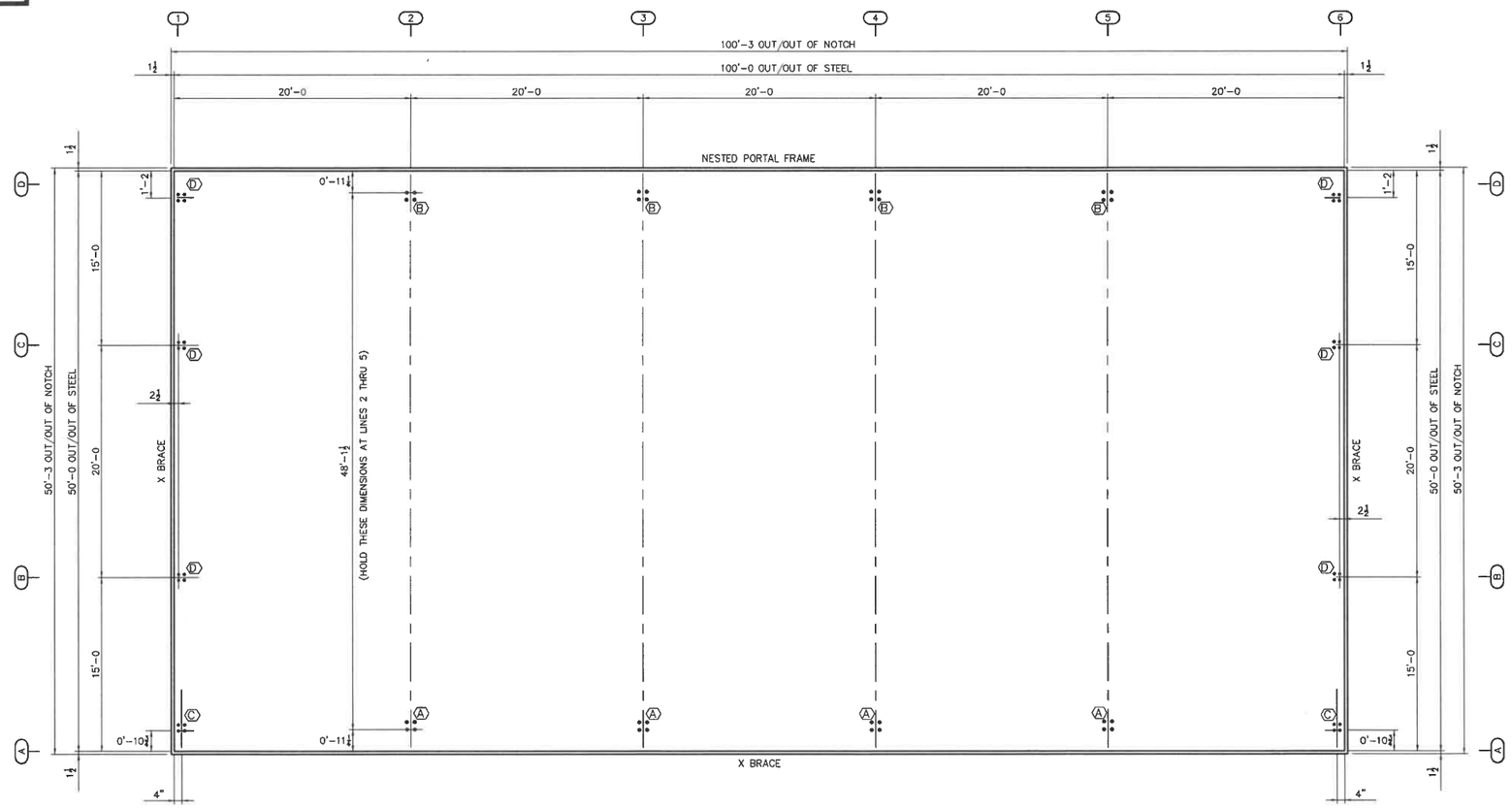
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:06:14 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

Anchor Rod Drawings

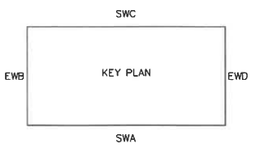
1) This drawing is for anchor rod placement only and is not foundation design.
2) Foundation must be square and level with all anchor rods true in size, location, and projection.
3) Projection shown must be held to keep threads clear of finished concrete.
4) This structural design data includes magnitude and location of design loads and support conditions, material properties, and type and size of major structural members necessary to show compliance with the Order Documents at the time of this issue. Any change to building loads or dimensions may change structural member sizes and locations shown. This structural design data will be superseded and voided by any future mailing.
5) Anchor rod size is determined by shear and tension at the bottom of the base plate. The length of the anchor rod and method of load transfer to the foundation are to be determined by the foundation engineer, and are not provided by the manufacturer.
6) Anchor rods are ASTM F1554 Gr. 36 material unless noted otherwise.
7) 3000 psi concrete compressive strength (fc) is assumed for the purpose of column base plate design unless otherwise noted.

FINISH FLOOR AT ELEVATION 100'-0



ANCHOR ROD SETTING PLAN

ANCHOR BOLTS TO BE DESIGNED BY FOUNDATION ENGINEER USING DIAMETERS SHOWN IN THIS TABLE.	
ANCHOR ROD DESCRIPTION	QUANTITY
⅞ "ø DIAMETER X	32
¾ "ø DIAMETER X	32
⅝ "ø DIAMETER X	8



MICHAEL W CUSTER

Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:06:27 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

Revision	Date	Description	By	Ck'd

Columbia, MS (662) 243-6400
Mount Pleasant, IA (319) 385-8001
Rocky Mount, NC (252) 977-2131
www.cecobuildings.com

CECO Building Systems

Project Name & Location:
STEEL DASH INC. DBA SOUTHERN STEEL, LLC
19121 YOUNG ST
FAIRHOPE, AL 36532-1619 US

Customer:
STEEL DASH INC. DBA SOUTHERN STEEL ST
2210 MAIN ST STE G
CHINO, CA 91710-4546 US
ATTN: CORNELIUS LINDA

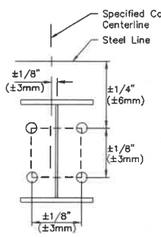
Drawing Status:
☐ Preliminary (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation (Not For Construction)

Scale:	NOT TO SCALE
Drawn by:	ALN 2/3/23
Checked by:	ABE 2/7/23
Project Engineer:	JDM
Job Number:	19-B-27589-1
Sheet Number:	F1 of 3

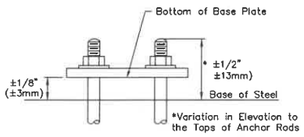
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only.The undersigned engineer is not the overall engineer of record for this project.

M.W. Custer, P.E.
Alabama P.E. PE21880

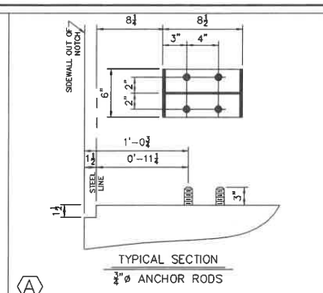
AISC CODE OF STANDARD PRACTICE TOLERANCES FOR SETTING ANCHOR RODS



ANCHOR ROD SETTING TOLERANCES

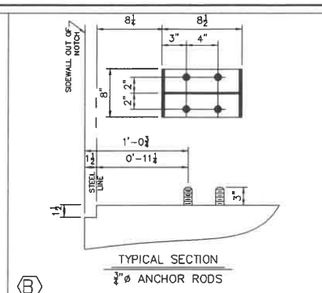


BASE PLATE AND ANCHOR ROD TOLERANCES



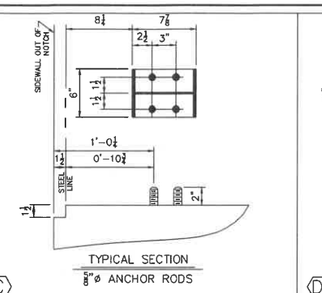
TYPICAL SECTION
¾"ø ANCHOR RODS

Ⓐ BASE OF STEEL AT ELEV. 100'-0



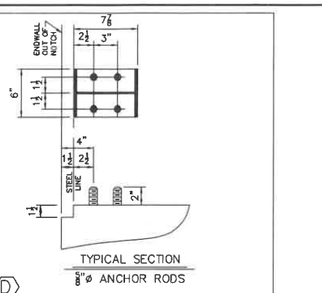
TYPICAL SECTION
¾"ø ANCHOR RODS

Ⓑ BASE OF STEEL AT ELEV. 100'-0



TYPICAL SECTION
⅞"ø ANCHOR RODS

Ⓒ BASE OF STEEL AT ELEV. 100'-0



TYPICAL SECTION
⅞"ø ANCHOR RODS

Ⓓ BASE OF STEEL AT ELEV. 100'-0

Revision	Date	Description	By	Ck'd

CECO Building Systems
Columbia, MS. (662) 240-8400
Mount Pleasant, M. (719) 3856001
Rocky Mount, NC. (252) 9774131
www.cecobuildings.com

Customer:
MAD DASH INC. DBA SOUTHERN STEEL ST
2210 MAIN ST STE G #146 US
ATTN: CORNELIUS LINDA

Project Name & Location:
MAD DASH INC. DBA SOUTHERN STEEL ST
19121 YOUNG ST
FAIRHOPE, AL 36532-1619 US

Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

Scale:	NOT TO SCALE
Drawn by:	ALN 2/3/23
Checked by:	ABE 2/7/23
Project Engineer:	JDM
Job Number:	19-B-27589-1
Sheet Number:	F2 of 3

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M.W. Custer, P.E.
Alabama P.E. PE21880

MICHAEL W CUSTER

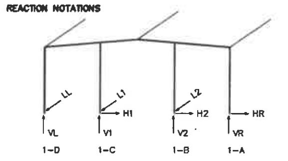
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:06:40 -06'00'

ALABAMA LICENSED No. 21880 PROFESSIONAL ENGINEER M.W. CUSTER

FRAME DESCRIPTION: Endwall EWB | USER NAME: Jose.Mar | DATE: 02/01/23 | PAGE: EW-1
JOB NAME: 27589A | FILE: REW3BLDG1
PATH: R:\jobs\Active\Eng\19-B-27589\ver01-jose.martinezjimenez\BLDG-A\run0

SUPPORT REACTIONS FOR EACH LOAD GROUP
NOTE: All reactions are in kips and kip-ft. TIME: 08:05:04

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	1-D			1-C			1-B			1-A		
LOAD GROUP	HL	VL	LL	H1	V1	L1	H2	V2	L2	HR	VR	LR
D	0.	0.5	0.	0.	1.0	0.	0.	1.0	0.	0.0	0.4	0.
C	0.	0.0	0.	0.	0.1	0.	0.	0.1	0.	0.0	0.0	0.
L	0.	1.2	0.	0.	3.9	0.0	0.	3.9	0.0	0.0	1.2	0.
W+	0.	-4.9	2.8	0.	-14.5	6.6	0.	-14.6	6.6	0.1	-4.9	0.
W-	0.	-4.9	-3.2	0.	-14.5	-7.0	0.	-14.6	-7.0	0.1	-4.9	0.
WR	0.	-4.9	0.0	0.	-11.5	0.0	3.2	-17.6	0.0	0.1	-4.9	0.
WL	0.	-4.9	0.0	-3.2	-17.6	0.0	0.	-11.5	0.0	0.1	-4.9	0.
E+	0.	0.	0.0	0.	0.	0.0	0.	0.	0.0	0.	0.	0.
E-	0.	0.	0.0	0.	0.	0.0	0.	0.	0.0	0.	0.	0.
ER	0.	0.	0.	0.	0.1	0.	0.1	-0.1	0.	0.	0.	0.
EL	0.	0.	0.	-0.1	-0.1	0.	0.	0.1	0.	0.	0.	0.

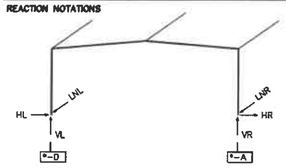
LOAD GROUP DESCRIPTION

D : Dead load
C : Collateral load
L : Live load
W+ : Wind load as an inward acting pressure
W- : Wind load as an outward acting suction
WR : Wind force from the right
WL : Wind force from the left
E+ : Seismic force acting inward
E- : Seismic force acting outward
ER : Seismic force from right
EL : Seismic force from left

FRAME ID #25 | USER NAME: Jose.Martinez | DATE: 01/31/23 | PAGE: 25-3
cs 50./16./20. 20./160./0. | JOB NAME: 27589A | FILE: frames_2-5.fra

SUPPORT REACTIONS FOR EACH LOAD GROUP
*LOCATION: Gridlines: 2 3 4 5
NOTES: (1) All reactions are in kips and kip-ft. TIME: 16:05:50
(2) The seismic overstrength factor (Omega) is not included in the "RBDWEQ" and "RBUPEQ" Load Group reactions. Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500
(3) Primary wind load cases are not concurrent.
(4) X-bracing reactions (RBPULW and RBUPEQ) are combined with LWL and LEQ groups only.

REACTION NOTATIONS



LOAD GROUP REACTION TABLE GRIDLINES * = 2 3 4 5

COLUMN	*-D			*-A		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
DL	0.6	2.4	0.0	-0.6	1.9	0.0
LL	2.3	6.0	0.0	-2.3	6.0	0.0
COLL	0.1	0.2	0.0	-0.1	0.3	0.0
RBDWEQ	0.0	0.4	0.2	-0.0	0.4	0.0
EQ	-0.1	-0.1	0.0	-0.1	0.1	0.0
RBUPEQ	0.0	-0.4	-0.2	-0.0	-0.4	-0.4
WL1	-9.5	-24.2	0.0	0.4	-17.7	0.0
WL2	-8.4	-2.9	0.0	-0.8	3.6	0.0
WL3	-0.2	-17.7	0.0	9.3	-24.2	0.0
WL4	0.9	3.6	0.0	8.2	-2.9	0.0
LWL1	-1.4	-22.4	0.0	1.9	-19.6	0.0
RBUPLW	0.0	-5.6	-3.3	-0.0	-6.0	-6.7
LWL2	-1.9	-19.6	0.0	1.4	-22.4	0.0
LWL3	-0.2	-1.0	0.0	0.7	1.8	0.0
LWL4	-0.8	1.8	0.0	0.2	-1.0	0.0
RBDWLW	0.0	5.6	3.3	-0.0	6.0	0.0

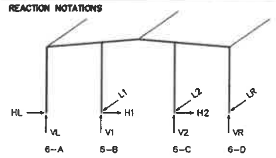
LOAD GROUP DESCRIPTION

DL : Roof Dead Load
LL : Roof Live Load
COLL : Roof Collateral Load
RBDWEQ : Downward Acting Rod Brace Load from Long. Seismic
EQ : Lateral Seismic Load [parallel to plane of frame]
RBUPEQ : Upward Acting Rod Brace Load from Long. Seismic
WL1 : Wind from Left to Right with +GCpi
WL2 : Wind from Left to Right with -GCpi
WL3 : Wind from Right to Left with +GCpi
WL4 : Wind from Right to Left with -GCpi
LWL1 : Windward Corner Left with +GCpi
RBUPLW : Upward Acting Rod Brace Load from Long. Wind
LWL2 : Windward Corner Right with +GCpi
LWL3 : Windward Corner Left with -GCpi
LWL4 : Windward Corner Right with -GCpi
RBDWLW : Downward Acting Rod Brace Load from Long. Wind

FRAME DESCRIPTION: Endwall EWD | USER NAME: Jose.Mar | DATE: 02/01/23 | PAGE: EW-2
JOB NAME: 27589A | FILE: REW4BLDG1
PATH: R:\jobs\Active\Eng\19-B-27589\ver01-jose.martinezjimenez\BLDG-A\run0

SUPPORT REACTIONS FOR EACH LOAD GROUP
NOTE: All reactions are in kips and kip-ft. TIME: 08:05:04

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN	6-A			6-B			6-C			6-D		
LOAD GROUP	HL	VL	LL	H1	V1	L1	H2	V2	L2	HR	VR	LR
D	0.0	0.4	0.	0.	1.0	0.	0.	1.0	0.	0.	0.5	0.
C	0.0	0.0	0.	0.	0.1	0.	0.	0.1	0.	0.	0.0	0.
L	0.0	1.2	0.	0.	3.9	0.0	0.	3.9	0.0	0.	1.2	0.
W+	-0.1	-4.9	0.	0.	-14.6	6.6	0.	-14.5	6.6	0.	-4.9	2.8
W-	-0.1	-4.9	0.	0.	-14.6	-7.0	0.	-14.5	-7.0	0.	-4.9	-3.2
WR	-0.1	-4.9	0.	0.	-11.5	0.0	3.2	-17.6	0.0	0.	-4.9	0.0
WL	-0.1	-4.9	0.	-3.2	-17.6	0.0	0.	-11.5	0.0	0.	-4.9	0.0
E+	0.	0.	0.	0.	0.	0.0	0.	0.	0.0	0.	0.	0.0
E-	0.	0.	0.	0.	0.	0.0	0.	0.	0.0	0.	0.	0.0
ER	0.	0.	0.	0.	0.1	0.	0.1	-0.1	0.	0.	0.	0.
EL	0.	0.	0.	-0.1	-0.1	0.	0.	0.1	0.	0.	0.	0.

LOAD GROUP DESCRIPTION

D : Dead load
C : Collateral load
L : Live load
W+ : Wind load as an inward acting pressure
W- : Wind load as an outward acting suction
WR : Wind force from the right
WL : Wind force from the left
E+ : Seismic force acting inward
E- : Seismic force acting outward
ER : Seismic force from right
EL : Seismic force from left

NOTES

1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.

2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE).

a) A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.

b) RIGID FRAMES

(1) GABLED BUILDINGS

(a) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING.

(b) INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE.

(2) SINGLE SLOPE BUILDINGS

(a) LEFT COLUMN IS THE LOW SIDE COLUMN.

(b) RIGHT COLUMN IS THE HIGH SIDE COLUMN.

(c) INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE.

c) ENDWALLS

(1) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.

(2) INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.

d) ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.

e) ANCHOR RODS ARE ASTM F1554 Gr. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.

f) X-BRACING

(1) ROD BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN IN THE REACTION TABLES.

(2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ AND RBDWEQ) DO NOT INCLUDE THE AMPLIFICATION FACTOR, Ω_0.

(3) FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ & RBDWEQ) ARE MULTIPLIED BY FORCE REDUCTION FACTOR, Rd, WHEN SPECIFIED SHORT-PERIOD SPECTRAL ACCELERATION RATIO $I_E F_a S_a(0.2)$ IS GREATER THAN 0.45.

3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.

a) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2015 IBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0.

b) FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL AND DO NOT CONTAIN THE RHO FACTOR.

c) FOR NBCC CODES, THE SEISMIC REACTIONS PROVIDED DO NOT CONTAIN THE R_d*R_o FACTOR.

THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

Revision	Date	Description	By	Ck'd

CECO Building Systems
Columbus, MS. (662) 243-6400
Mount Pleasant, IA. (319) 385-8001
Rocky Mount, NC. (252) 977-2131
www.cecobuildings.com

Customer:
MAD DASH INC. DBA SOUTHERN STEEL ST
2210 MAIN ST STE G
DAPHNE, AL 36526-4546 US
ATTN: CORNELIUS LINDA

Project Name & Location:
MAD DASH INC. DBA SOUTHERN STEEL ST
19121 YOUNG ST
FAIRHOPE, AL 36532-1619 US

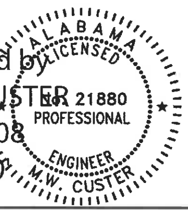
Drawing Status:
☐ Preliminary (Not For Construction)
☐ For Approval (Not For Construction)
☒ For Construction Permit
☐ For Erector Installation

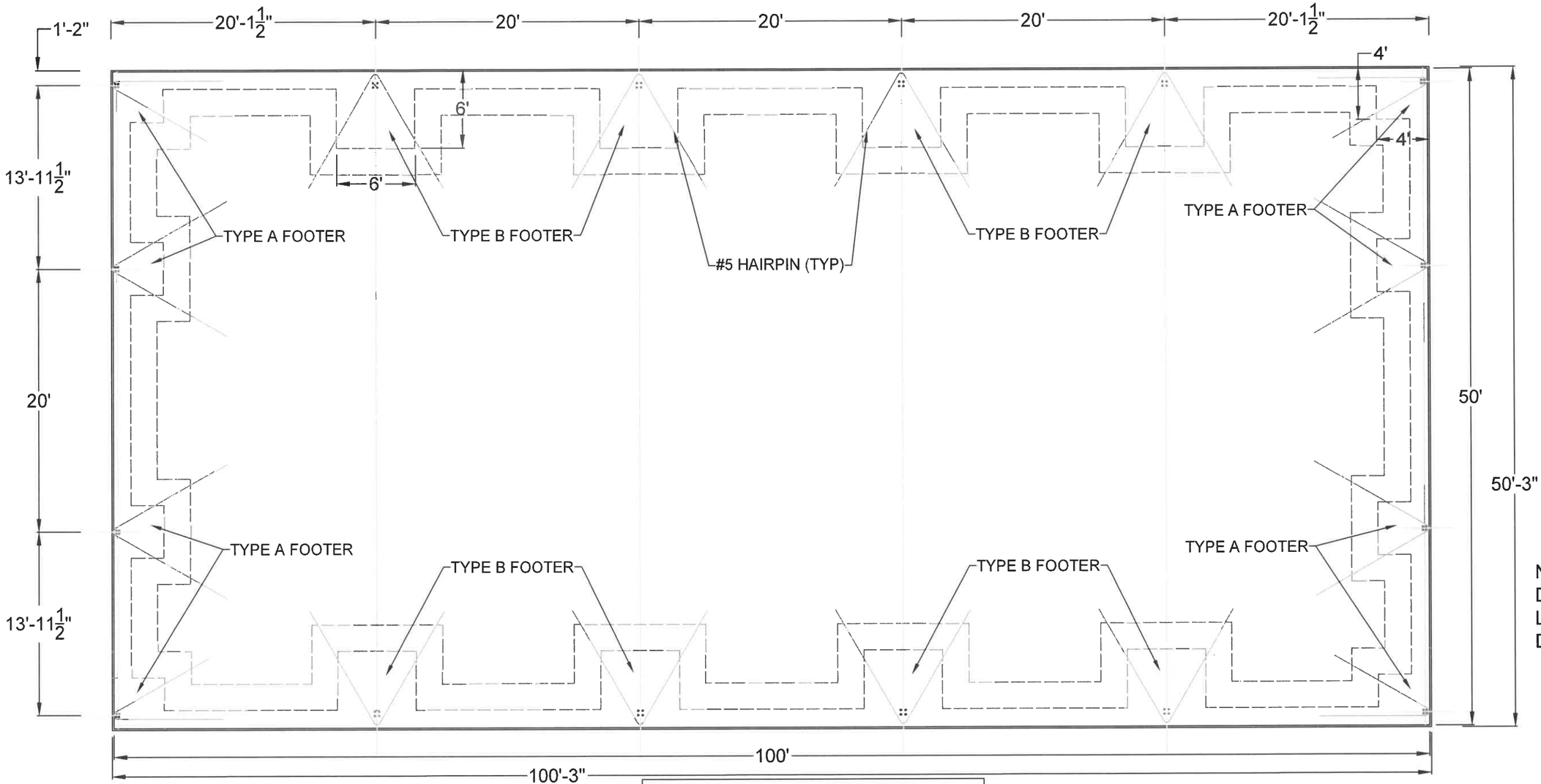
Scale: NOT TO SCALE
Drawn by: ALN 2/3/23
Checked by: ABE 2/7/23
Project Engineer: JDM
Job Number: 19-B-27589-1
Sheet Number: F3 of 3

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

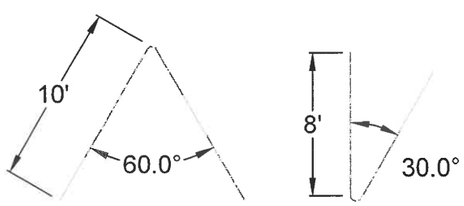
M.W. Custer, P.E.
Alabama P.E. PE21880

MICHAEL W CUSTER
Digitally signed by MICHAEL W CUSTER
Date: 2023.02.08 12:06:54 -06'00'





NOTE: SEE CECO DRAWING FOR A.B. LAYOUT & DETAIL DIMENSIONS



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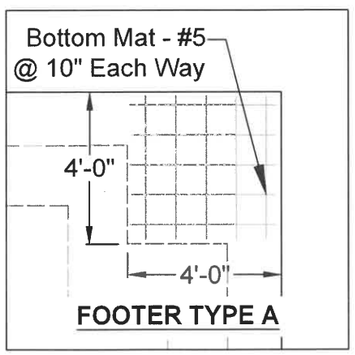
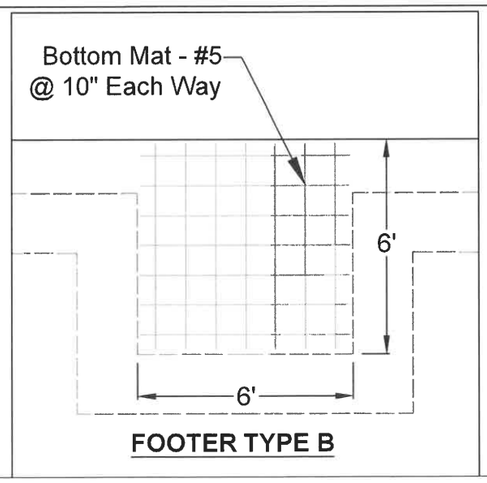
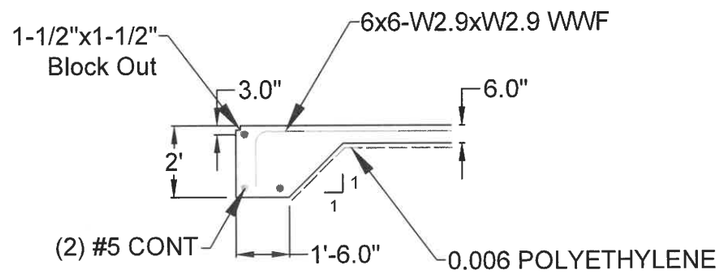


FOOTER TYPE A



FOOTER TYPE B

FOOTING AT SLAB PERIMETER



CITY OF FAIRHOPE - NEW ELECTRIC TRUCK SHED

RFP - Concrete Slab Plan and Details

Drawn By:	RDJ
Project No.:	2023-PWI 007S
Date:	2/9/2023
Scale:	N.T.S.

Model 2500

Last updated: 09.04.13

Part 1- GENERAL

1.01 DESCRIPTION—

1.01.01 Type: Continuous sheet rolling door Model 2500 as manufactured by Janus International, Temple, GA. Available in sizes up to 18'0" x 18'0".

1.01.02. Mounting: To be interior or exterior face mounted on a prepared jamb.

1.01.03. Related Work: Preparation of opening, miscellaneous or structural steel, iron work, access panels, master keying cylinders, finish or field painting, electrical wiring, conduit, disconnecting switches are in the scope of the work of other sections or trades.

1.02 QUALITY ASSURANCE— Qualifications of Manufacturer: Products utilized in this section shall be manufactured by an organization who regularly engages in the production of similar products and has a proven history of successful manufactured products acceptable to the Architect, such as Janus International.

1.03 GUARANTEE— All doors and components specified herein shall be guaranteed to be free of workmanship and defect for a period of 1 year.

Part 2- PRODUCT

2.01 CURTAIN—

2.01.01 Sheets: Continuous 20" corrugated sheets roll formed from 26 gauge ASTM A653 Grade 80 full hard steel and lock seamed together.

2.01.02 Finish: galvanized and pre-painted with long-lasting Super Durable Polyester paint guaranteed with a 40 year film integrity warranty to not crack, peel, flake, split, delaminate or blister. Additional guarantee up to 25 years against fading or changing color based on color chosen.

2.01.03 Bottom Bar: Roll formed clear acrylic coated galvanized steel reinforced with a 2" x 1-1/2" - 12 gauge galvanized angle that extends fully into the guides.

2.01.04 1-1/2" wide nylon strips attached on the edge of each end of both the front and back of the curtain to control stretch and reduce wear.

2.02 WEATHERSTRIPPING—

2.02.01 Black PVC bulb-type astragal affixed to the bottom bar assembly provides positive contact with the floor.

2.02.02 (Optional) Side draft stop attaching to guide leg.

2.02.03 (Optional) Black flexible neoprene top draft stop with 2" lip attached to curtain.

2.02.04 (Optional) 4" Header seal attached to header.

2.03 BARREL ASSEMBLY—Galvanized coil steel fabricated in a 12" diameter spiral formation to enclose spring counterbalance system and provide full span curtain weight support. Attached galvanized drums are furnished with grease-filled, shielded radial ball bearings at rotating points around the axle.

2.04 SPRING COUNTERBALANCE— Factory lubricated, oil tempered, helical torsion springs located inside the barrel made of wire conforming to ASTM A229. Springs are attached to the steel axle tube by means of a welded spring clip. Axle tube provided is sufficient size to carry curtain load and spring torque. Spring cycle life of 15,000.

2.05 SUPPORT BRACKETS— Galvanized and reinforced 12 gauge formed steel brackets.

2.06 SPRING TENSIONER—External mounted cam action tension retaining device allows for field adjustment of spring tension on all springs.

2.07 GUIDE ASSEMBLY—Universal mounted guides rolled formed from 12 gauge galvanized steel. 3" guide depth furnished for sufficient curtain engagement. Removable galvanized door stop at top of each guide.

2.08 OPERATION—

2.08.01 Hand operated with #6 Angola rope on size up to 10'0" x 10'0" attaching to the bottom bar assembly.

2.08.02 Universal 5.7:1 cast iron reduced drive hand operated chain hoist furnished for all doors over 10'0" wide or 10'0" tall.

2.08.03 (Optional) Electric operator (furnished by vendor) with electric 72 tooth sprocket operator kit for door adaptation.

2.09 LOCKING MECHANISM—Dual steel bottom bar slide locks suitable for pad locks (provided by others) mount to the inside angle of the bottom bar. Chain keeper guide mounted to wall for chain operation doors.

2.10 HOOD (Optional) — Fabricated from 20 gauge steel and reinforced with end caps and roll formed edges. Manufactured square.

2.11 FINISH—Non-galvanized surfaces, excluding axle tube, to consist of shop coat of rust inhibitor primer.

Part 3- EXECUTION

3.01 INSTALLATION— To be performed by an authorized Janus International representative or professional door installer in accordance with the Janus installation standards, instructions and recommendations.

HEAD ROOM REQUIMENTS

Opening Height	Vertical Headroom	Horizontal Headroom
Thru 8'0"	20"	20"
Over 8'0" thru 10'0"	21"	21"
Over 10'0" thru 14'0"	21 ½ "	21"
Over 14'0" thru 16'0"	22"	21"
Over 16'0" thru 18'0"	22"	22"
Over 18'0" thru 20'0"	22"	22"

SIDE ROOM REQUIREMENTS

Operation	Guide	Outside of Bracket Tensioner End	Outside of Bracket Drive End	Outside of Hand Chain Drive	Each End of Axle
Push Up	4"	6"	6"	---	8-1/2"
Reduced Drive Chain	4"	6"	6-3/4"	9-3/4"	8-1/2"
**Electric	4"	6"	6-3/4"	---	8-1/2"

*Vertical head room: the space above the clear opening on the same face of wall (header).

*Horizontal head room: the amount of space required off of the wall to which the door is fastened.

*Side room: the amount of space required on each side away from the opening along the face of the wall.

**Excludes electric operation.

CITY OF FAIRHOPE, ALABAMA

NON-MANDATORY PRE-BID MEETING

Meeting Date: 11/14/24 at 10:00 a.m.

Bid No. 25-005-2025-PWI-014 Metal Building – 50' x 120' Pre-engineered Metal Building with Foundation Electric Storage Building Addition for the City of Fairhope – Electric Utility

Representative's Name	Company	Phone	Email
Erin Wolfe	City of Fairhope – Purchasing Dept.	251-279-6231	Erin.Wolfe@FairhopeAL.gov
Rhonda Cunningham	City of Fairhope – Purchasing Dept.	251-990-0118	Rhonda.Cunningham@FairhopeAL.gov
Richard Johnson	COF – City Engineer	251-929-0360	Richard.Johnson@FairhopeAL.gov
George Ladd	COF – Public Works Director	251-928-8003	George.Ladd@FairhopeAL.gov
~~John Thomas~~	~~COF – Assistant Public Works Director~~	~~251-928-8003~~	~~John.Thomas@FairhopeAL.gov~~
~~David Thomas~~	~~COF – ROW Inspector~~	~~251-928-8003~~	~~David.Thomas@FairhopeAL.gov~~
Ben Patterson	COF - Electric Department	251-928-8003	Ben.Patterson@FairhopeAL.gov
Cona Billyzone	Green-Simmons Co	850 429-0134	cona@green-simmons.com
Lane Otto	Klasman & Day GC	251-376-0955	lotto@kd-gc.com
William Morgan	Kemko Inc	251-626-0595	billy@kemkobuildings.com
Dustin Helton	Brotherhood Service Co	601 770 0481	dustin.helton@brotherhoodservice.co
Scott Fitzpatrick	Merit Building Systems	251-404-4005	sfitzpatrick@meritbuildings.com
Matt Reed	Reed-Hays Construction	251-217-4996	mreed@reedhaysconstruction.com