1. GENERAL REQUIREMENTS:

1.1. THESE STRUCTURAL DRAWINGS HAVE BEEN PREPARED IN ACCORDANCE WITH THE SPECIFIED BUILDING CODE. ALL CONSTRUCTION SHALL CONFORM TO THE EDITION OF THE BUILDING CODE REFERENCED. REFERENCE TO OTHER SPECIFICATIONS OR CODES SHALL MEAN THE VERSION INDICATED IN THE BUILDING CODE.

1.2. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS ARE A PORTION OF THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR AND SUBCONTRACTORS SHALL REFERENCE AND COORDINATE WITH ALL OTHER DISCIPLINES DRAWINGS. ANY DISCREPANCIES OR OMISSIONS SHALL BE REPORTED TO THE ARCHITECT/ENGINEER.

1.3. THE CONTRACTOR SHALL VERIFY SITE CONDITIONS AND COORDINATE STRUCTURAL DIMENSIONS, ELEVATIONS, AND SECTIONS WITH ARCHITECTURAL DIMENSIONS, ELEVATIONS, AND SECTIONS, AND REPORT ANY DISCREPANCY TO THE ARCHITECT/ENGINEER PRIOR TO THE FABRICATION OR INSTALLATION OF STRUCTURAL MEMBERS.

1.4. STRUCTURAL DRAWINGS SHOW TYPICAL AND CERTAIN SPECIFIC CONDITIONS ONLY AND SHALL APPLY FOR LIKE OR SIMILAR CONDITIONS UNLESS NOTED OTHERWISE. FOR CONDITIONS NOT SPECIFICALLY SHOWN, PROVIDE DETAILS SIMILAR TO THOSE SHOWN. IF THERE IS A QUESTION REGARDING THE APPLICABILITY OF A DETAIL, CONTACT THE ARCHITECT/ENGINEER IN WRITING REQUESTING CLARIFICATION.

1.5. COORDINATE AND VERIFY ALL OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL, PLUMBING, AND/OR ELECTRICAL DRAWINGS BEFORE PROCEEDING WITH CONSTRUCTION. STRUCTURAL DRAWINGS ONLY SHOW OPENINGS RELATIVE TO THE STRUCTURE.

1.6. COORDINATE ALL LIMITS AND DEPTHS OF DEPRESSIONS FOR FLOOR FINISHES WITH ARCHITECTURAL DRAWINGS AND SCHEDULES. LIMITS SHOWN ON STRUCTURAL DRAWINGS ARE SCHEMATIC. COORDINATE FLOOR JOINTS WITH ARCHITECTURAL FLOOR FINISHES.

1.7. STRUCTURAL MEMBERS SHALL NOT BE CUT, NOTCHED, CHANGED, OR MODIFIED WITHOUT THE WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.

1.8. DO NOT SCALE FOR DIMENSIONS NOT SHOWN ON THE DRAWINGS. SEND A WRITTEN REQUEST FOR INFORMATION TO THE ARCHITECT/ENGINEER FOR DIMENSIONS NOT PROVIDED.

1.9. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. UNLESS OTHERWISE INDICATED, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. CONTRACTOR IS RESPONSIBLE FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.

1.10. THE STRUCTURE SHOWN ON THESE DRAWINGS IS STRUCTURALLY SOUND ONLY IN ITS COMPLETED FORM. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. THE ENGINEER WILL NOT ADVISE ON OR ISSUE DIRECTION RELATED TO SAFETY REQUIREMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL APPLICABLE OSHA REGULATIONS.

1.11. CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED FLOORS/ROOFS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT CONSTRUCTION LOADS DO NOT EXCEED THE DESIGN LIVE LOAD.

1.12. DISSIMILAR METALS MUST BE SEPARATED BY A COATING SUCH AS ECK CORROSION COATING OR AN APPROVED EQUIVALENT, OR NEOPRENE GASKET MATERIAL TO PREVENT GALVANIC ACTION.

1.13. THE CONTRACTOR IS RESPONSIBLE FOR LOCATING ALL UNDERGROUND UTILITIES IN THE AREA OF CONSTRUCTION THAT MIGHT BE AFFECTED BY, OR OTHERWISE INTERFERE WITH, INSTALLATION OF NEW WORK. THIS INCLUDES THOSE THAT MIGHT BE DAMAGED BY NEW FOUNDATIONS OR OTHER WORK, AND THOSE WHOSE PRESENCE MIGHT LEAD DAMAGE TO THE NEW WORK (e.g. DIFFERENTIAL SETTLEMENT).

2. EXISTING CONDITIONS:

2.1. EXISTING CONDITIONS DEPICTED ON THESE DRAWINGS ARE TO BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO FABRICATION OR CONSTRUCTION. IN THE EVENT EXISTING CONDITIONS ARE DIFFERENT THAN SHOWN ON THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER IMMEDIATELY.

2.2. CONTRACTOR SHALL FIELD VERIFY CONDITIONS DEPICTED ON THESE DRAWINGS AS THEY ARE UNCOVERED DURING CONSTRUCTION. IN THE EVENT EXISTING CONDITIONS ARE DIFFERENT THAN SHOWN ON THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER IMMEDIATELY.

2.3. DIMENSIONS RELATIVE TO AN EXISTING STRUCTURE ARE APPROXIMATE AND SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO MATERIALS PURCHASE, FABRICATION, OR CONSTRUCTION. CONTRACTOR SHALL NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER IN WRITING OF DISCREPANCIES.

2.4. FIELD DIMENSIONS VERIFIED BY THE CONTRACTOR SHALL BE SHOWN ON THE SUBMITTED SHOP DRAWINGS.

2.5. IT IS THE CONTRACTORS RESPONSIBILITY TO PROVIDE ALL NECESSARY BRACING, SHORING AND OTHER SAFEGUARDS TO MAINTAIN ALL PARTS OF THE EXISTING WORK IN A SAFE AND UNDAMAGED CONDITION DURING THE PROCESS OF DEMOLITION AND NEW CONSTRUCTION.

3. DESIGN CRITERIA:

3.1. GENERAL BUILDING CODE:

3.1.1. INTERNATIONAL BUILDING CODE, IBC 2018 EDITION. ALL CODES BELOW ARE THE EDITION REFERENCED IN THE IBC.

3.2. DESIGN LOAD CRITERIA:

3.2.1. MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, AMERICAN SOCIETY OF CIVIL ENGINEERS, ASCE 7.

3.3. CONCRETE:

3.3.1. BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, AMERICAN CONCRETE INSTITUTE, ACI 318.

3.4. STRUCTURAL STEEL:

3.4.1. SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, AMERICAN INSTITUTE OF STEEL CONSTRUCTION, AISC 360.

3.5. TIMBER:

3.5.1. NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, AMERICAN FOREST & PAPER ASSOCIATION/AMERICAN WOOD COUNCIL, NDS.

4. DESIGN LOADS:

4.1. DESIGN DEAD LOAD IS ACTUAL WEIGHT OF THE STRUCTURE. ANY CHANGES IN CONSTRUCTION MATERIALS FROM THOSE SHOWN ON THE ARCHITECTURAL OR STRUCTURAL DRAWINGS SHALL BE REPORTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF LOAD-CARRYING CAPACITY OF THE STRUCTURE.

4.2. LIVE LOADS (PSF):

4.3. A PARTITION LIVE LOAD OF 15 PSF HAS BEEN ADDED WHERE APPLICABLE.

4.4. LIVE LOAD REDUCTIONS HAVE BEEN APPLIED IN ACCORDANCE WITH THE BUILDING CODE WHEN PERMITTED.

4.2. WIND LOADS:

DESIGN WIND SPEED (V)	157 MPH
ALLOWABLE WIND SPEED (Vasd)	122 MPH
RISK CATEGORY	II
EXPOSURE CATEGORY	B
PRESSURE COEFFICIENT (RESTROOM)	+/- 0.18
PRESSURE COEFFICIENT (PAVILIONS/KIOSK/ENTRY SIGN)	0.00

SEE DRAWINGS FOR EXTERIOR COMPONENT AND CLADDING WIND PRESSURES.

4.7. SEISMIC LOADS (RESTROOM):

IMPORTANCE FACTOR (Ie) 1.00
SOIL SITE CLASS D
MAPPED SPECTRAL RESPONSE ACCEL.:
Ss=0.090
S1=0.058
DESIGN SPECTRAL RESPONSE ACCEL.:
Sds=0.096
Sd1=0.093
SEISMIC DESIGN CATEGORY B
SEISMIC RESPONSE COEFFICIENT (Cs) 0.064
RESPONSE MODIFICATION FACTOR (R) 1.5
DESIGN BASE SHEAR 0.064*Ws KIPS
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE METHOD
BASIC SEISMIC-FORCE-RESISTING SYSTEM: ORDINARY PLAIN CONCRETE SHEAR WALLS

4.7. SEISMIC LOADS (PAVILIONS):

IMPORTANCE FACTOR (Ie) 1.00
SOIL SITE CLASS D
MAPPED SPECTRAL RESPONSE ACCEL.:
Ss=0.090
S1=0.058
DESIGN SPECTRAL RESPONSE ACCEL.:
Sds=0.096
Sd1=0.093
SEISMIC DESIGN CATEGORY B
SEISMIC RESPONSE COEFFICIENT (Cs) 0.048
RESPONSE MODIFICATION FACTOR (R) 1.5
DESIGN BASE SHEAR 0.048*Ws KIPS
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE METHOD
BASIC SEISMIC-FORCE-RESISTING SYSTEM: STEEL ORDINARY MOMENT FRAMES

5. STRUCTURAL OBSERVATIONS:

5.1. STRUCTURAL OBSERVATIONS ARE VISUAL OBSERVATIONS BY A STRUCTURAL ENGINEER OF THE IN-PLACE STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS AT THE TIME OF THE OBSERVATION.

5.2. STRUCTURAL OBSERVATIONS ARE REQUIRED FOR THIS PROJECT IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17 OF THE BUILDING CODE. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER, PER THE SCHEDULE PROVIDED IN THE CONSTRUCTION DOCUMENTS, WHEN SUCH ITEMS HAVE PROGRESSED TO THE POINT WHERE THEY WILL BE IN PLACE AND READY FOR OBSERVATION. FAILURE TO NOTIFY MAY REQUIRE REMOVAL OF COMPLETED CONSTRUCTION.

6. SPECIAL INSPECTIONS:

6.1. SPECIAL INSPECTIONS ARE REQUIRED FOR THIS PROJECT IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17 OF THE BUILDING CODE. AN APPROVED SPECIAL INSPECTOR WITH QUALIFICATIONS SATISFACTORY TO THE BUILDING OFFICIAL SHALL PERFORM THE REQUIRED SPECIAL TESTS AND INSPECTIONS.

6.2. OBSERVATION BY THE STRUCTURAL ENGINEER'S OFFICE DOES NOT REPLACE TESTING AND INSPECTIONS BY THE TESTING AGENCY OR THE SPECIAL INSPECTOR.

6.3. THE COSTS OF THE SPECIAL INSPECTOR'S SERVICES SHALL BE PAID FOR BY THE OWNER. THE COSTS OF OTHER INSPECTIONS AND TESTING SPECIFIED IN THE CONTRACT DOCUMENTS SHALL BE PAID FOR BY THE CONTRACTOR.

6.4. THE FOLLOWING DOCUMENTS HAVE BEEN PREPARED FOR THIS PROJECT AS A PART OF THESE CONSTRUCTION DOCUMENTS:

6.4.1. STATEMENT OF SPECIAL INSPECTIONS

6.4.2. SCHEDULE OF SPECIAL INSPECTIONS

6.4.3. STATEMENT OF SPECIAL INSPECTIONS REQUIREMENTS FOR WIND RESISTANCE.

6.5. CONTRACTOR AND SUBCONTRACTORS ENGAGED IN CONSTRUCTION OF MAIN WIND FORCE OR SEISMIC FORCE RESISTING SYSTEMS SHALL SUBMIT A STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND OWNER IN ACCORDANCE WITH THE PROVISIONS OF CHAPTER 17 OF THE BUILDING CODE.

6.6. THE CONTRACTOR SHALL COORDINATE THE INSPECTION SERVICES IN ACCORDANCE WITH PROGRESS OF THE WORK. THE CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE TO THE INSPECTOR TO ALLOW PROPER SCHEDULING OF PERSONNEL.

6.7. ALL REPORTS AND SHOP CERTIFICATIONS OF SPECIAL INSPECTIONS TO BE PERFORMED ON THE PREMISES OF A FABRICATOR'S SHOP SHALL BE SUBMITTED TO THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DISTRIBUTING THESE REPORTS TO THE SPECIAL INSPECTOR, THE ARCHITECT, AND THE STRUCTURAL ENGINEER IN A TIMELY MANNER.

6.8. THE SPECIAL INSPECTOR SHALL PREPARE THE REQUIRED QUALITY ASSURANCE PLANS AND SUBMIT THE PLAN TO THE BUILDING OFFICIAL, ARCHITECT, AND STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION.

6.9. ALL SPECIAL INSPECTION REPORTS SHALL BE PREPARED BY AND BEAR THE SEAL OF THE SPECIAL INSPECTOR. ALL REPORTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL, ARCHITECT, THE STRUCTURAL ENGINEER. THE FREQUENCY OF REPORTS SHALL BE AS AGREED UPON BY THE BUILDING OFFICIAL.

6.10. REPORTS SHALL INDICATE THAT THE WORK WAS PERFORMED AND CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. ALL NONCONFORMING ITEMS SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN IF UNCORRECTED, TO THE BUILDING OFFICIAL, ARCHITECT, AND THE STRUCTURAL ENGINEER.

6.11. THE SPECIAL INSPECTOR, UPON COMPLETION OF THE WORK AND PRIOR TO THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY, SHALL SUBMIT A SIGNED AND SEALED FINAL REPORT DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES IN THE PRIOR REPORTS.

7. SHOP DRAWINGS AND SUBMITTALS:

7.1. THE USE OR REPRODUCTION OF THE CONTRACT DRAWINGS BY ANY CONTRACTOR, SUBCONTRACTOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS IS NOT PERMITTED.

7.2. SHOP DRAWINGS SHALL DETAIL ALL CONDITIONS IN ACCORDANCE WITH SPECIFIED STANDARDS AND THE SPECIFIC REQUIREMENTS OF THIS PROJECT AS INDICATED ON THE DRAWINGS.

7.3. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS SPECIFIED IN THE CONTRACT DOCUMENTS. ALL SHOP DRAWINGS MUST BE REVIEWED AND "APPROVED" BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE STRUCTURAL ENGINEER. REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THEIR RESPONSIBILITIES.

7.4. SHOP DRAWINGS AND CALCULATIONS SUBMITTED AS PART OF A DELEGATED DESIGN SHALL BE SIGNED AND SEALED BY A LICENSED ENGINEER IN THE STATE OF THE PROJECT.

7.5. HARDCOPY SHOP DRAWING SUBMITTALS: SUBMIT ALL SHOP DRAWINGS ON THREE PRINTS ONLY. ONE PRINT WILL BE RETURNED TO THE CONTRACTOR. ALL PRINTS REQUIRED BY THE CONTRACTOR ARE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE MADE AFTER APPROVED SHOP DRAWINGS ARE RETURNED. IF ADDITIONAL PRINTS ARE SUBMITTED, THEY WILL BE RETURNED UNMARKED.

7.6. ELECTRONIC SHOP DRAWING SUBMITTALS: SUBMIT ALL ELECTRONIC SHOP DRAWINGS IN PDF FORMAT. REVIEWED SHOP DRAWINGS WILL BE RETURNED IN PDF FORMAT. ALL PRINTS REQUIRED BY THE CONTRACTOR ARE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE MADE AFTER APPROVED SHOP DRAWINGS ARE RETURNED.

7.7. RESUBMITTED SHOP DRAWINGS: RESUBMITTED SHOP DRAWINGS SHALL HAVE ALL CHANGES SINCE THE PREVIOUS SUBMISSION IDENTIFIED BY CLOUDING OR OTHER CLEAR COMMUNICATION. RE-REVIEWED SHOP DRAWINGS WILL ONLY BE REVIEWED FOR IDENTIFIED CHANGES.

7.8. SHOP DRAWINGS: SEE THE RELATED MATERIAL SECTION FOR THE REQUIRED SUBMITTALS AND SHOP DRAWINGS.

8. SOILS, SLABS, WALLS, AND SHALLOW FOUNDATIONS:

8.1. THE FOUNDATION AND SLAB ON GRADE DESIGN IS BASED ON CRITERIA ESTABLISHED IN THE GEOTECHNICAL REPORT BY THOMPSON ENGINEERING TITLED "GEOTECHNICAL DESIGN REPORT PROJECT NO.22-1101-0229 DATED _________". THE CONTRACTOR SHALL OBTAIN A COPY OF THE GEOTECHNICAL REPORT FROM THE OWNER AND FOLLOW ALL REQUIREMENTS AND RECOMMENDATIONS.

8.2. MAX ALLOWABLE BEARING PER GEOTECHNICAL REPORT (PSF):

UNLESS NOTED OTHERWISE15008.03. ALL FOUNDATION BEARING SURFACES SHALL BE REVIEWED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE TO ENSURE THEIR COMPLIANCE WITH THE PRESSURES NOTED, THE REQUIREMENTS OF THE PROJECT SPECIFICATIONS, AND THE GEOTECHNICAL REPORT. IN THE ABSENCE OF SPECIFIC REQUIREMENTS, A DYNAMIC CONE PENETROMETER TEST (ASTM STP-399) SHALL BE PROVIDED AT EACH ISOLATED COLUMN FOOTING AND A MAXIMUM OF EVERY 50' OF CONTINUOUS FOUNDATION AND/OR THICKENED SLAB TO VERIFY BEARING CAPACITY. SOILS DEEMED UNSUITABLE SHALL BE UNDERCUT TO COMPETENT MATERIAL, BACKFILLED WITH AN APPROVED AND PROPERLY COMPACTED MATERIAL, AND RETESTED.

8.4. ALL FOOTING ELEVATIONS ARE ESTIMATED AND MAY BE ADJUSTED IN THE FIELD BY THE GEOTECHNICAL ENGINEER.

8.5. COMPACTED FILL SHALL MEET THE REQUIREMENTS NOTED IN THE GEOTECHNICAL REPORT.

8.6. WHEN EXCAVATIONS APPROACH THE GROUND WATER TABLE, THE WATER LEVEL SHALL BE LOWERED BY AN ACCEPTABLE DEWATERING SYSTEM SO THAT THE WATER LEVEL IS MAINTAINED CONTINUOUSLY A MINIMUM OF 2' BELOW THE EXCAVATION DURING CONSTRUCTION.

8.7. CONTRACTOR SHALL FOLLOW THE SITE WORK AND SUBGRADE RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL REPORT.

8.8. PROVIDE 4" COMPACTED GRANULAR FILL BENEATH ALL EARTH SUPPORTED SLABS. PROVIDE A MINIMUM 10 MIL VAPOR BARRIER BETWEEN BOTTOM OF SLAB AND TOP OF GRANULAR FILL. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR SPECIFIED VAPOR BARRIER THICKNESS.

8.9. PROVIDE ½" P.E.J FILLER AROUND PERIMETER OF SLABS WHERE THEY ABUT VERTICAL SURFACES AND AT COLUMN ISOLATION JOINTS AS DETAILED.

8.10. SEE PROJECT SPECIFICATIONS FOR FLOOR FLATNESS AND FLOOR LEVELNESS REQUIREMENTS.

8.11. BACKFILL FOR FOUNDATION AND RETAINING WALLS SHALL BE A FREE DRAINING GRANULAR MATERIAL. BACKFILL SHALL BE COMPACTED SUFFICIENTLY TO PREVENT SUBSIDENCE OF SURFACE ADJACENT TO WALL. THE GRANULAR MATERIAL SHALL BE PLACED IN A 45 DEGREE WEDGE EXTENDING FROM THE BASE OF THE FOOTING.

8.12. COORDINATE THROUGH WALL OR BEHIND WALL DRAINAGE SYSTEM WITH THE GEOTECHNICAL AND CIVIL ENGINEER.

8.13. HEAVY EQUIPMENT FOR SPREADING AND COMPACTING BACKFILL SHALL NOT BE OPERATED CLOSER TO WALL, GRADE BEAM, ETC., THAN A DISTANCE EQUAL TO THE HEIGHT OF BACKFILL ABOVE THE TOP OF WALL FOOTING AND BOTTOM OF GRADE BEAM, ETC. THE AREA REMAINING SHALL BE COMPACTED BY HAND TAMPERS.

8.14. FOUNDATIONS SHALL NOT BEAR DIRECTLY ON ROCK. UNDERCUT FOUNDATIONS A MINIMUM OF ONE FOOT AND BACKFILL WITH PROPERLY COMPACTED STRUCTURAL FILL PER THE GEOTECHNICAL REPORT.

8.15. SIDES OF FOUNDATIONS SHALL BE FORMED UNLESS CONDITIONS PERMIT EARTH FORMING.

8.16. HORIZONTAL BARS IN FOOTINGS AND STEM WALLS SHALL BE CONTINUOUS. PROVIDE CORNER BARS AT ALL INTERSECTIONS UNLESS NOTED OTHERWISE.

8.17. SUPPORT BOTTOM REINFORCING IN FOOTINGS WITH CONCRETE BRICKS OR PLASTIC CHAIRS SPACED A MAXIMUM OF 3'-0" EACH WAY. SUPPORTS SHALL BE POSITIONED TO PROVIDE A MINIMUM OF 3" CLEAR TO BOTTOM OF LOWEST REINFORCING BAR.

8.18. CONSTRUCTION JOINTS IN CONTINUOUS FOOTINGS SHALL BE FORMED VERTICALLY WITH A CLASS B LAP IN HORIZONTAL REINFORCING.

8.19. POUR A 2" MUD MAT OF LEAN CONCRETE IN THE BOTTOM OF A FOOTING EXCAVATION THAT WILL BE EXPOSED TO RAIN OR REMAIN OPEN OVERNIGHT.

8.20. ALL REINFORCING SHALL BE TIED IN PLACE PRIOR TO PLACING CONCRETE.

8.21. FOUNDATION PENETRATIONS SHALL BE SUBJECT TO APPROVAL BY THE STRUCTURAL ENGINEER.

8.22. WHERE FOOTING STEPS ARE REQUIRED, THEY SHALL BE NO STEEPER THAN ONE VERTICAL TO TWO HORIZONTAL.

8.23. WHERE GRAVITY PLUMBING LINES OCCUR BELOW TOP OF WALL FOOTING, STEP FOOTING DOWN TO PROVIDE CLEARANCE. COORDINATE WITH PLUMBING DRAWINGS FOR LOCATIONS, SIZES, AND INVERTS.

8.24. PROTECT STRUCTURES, UTILITIES, SIDEWALKS, PAVEMENTS, AND OTHER FACILITIES FROM DAMAGE CAUSED BY SETTLEMENT, LATERAL MOVEMENT, UNDERMINING, WASHOUT, AND OTHER HAZARDS CREATED BY EARTHWORK OPERATIONS.

8.25. PREVENT SURFACE WATER AND GROUND WATER FROM ENTERING EXCAVATIONS AND FROM PONDING ON PREPARED SUBGRADES AND SLABS. DO NOT USE FOUNDATION EXCAVATIONS AS TEMPORARY DRAINAGE DITCHES.

8.26. DEWATER EXCAVATIONS AND REMOVE ANY WET MATERIAL PRIOR TO THE PLACING OF CONCRETE.

8.27. IMMEDIATELY NOTIFY THE OWNERS REPRESENTATIVE AND ENGINEER IF UNUSUAL SOIL CONDITIONS ARE FOUND.

9. TIMBER PILES:

9.1. TIMBER PILE CONSTRUCTION SHALL COMPLY WITH THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE, AMERICAN WOOD COUNCIL, AND ASTM D25 STANDARD SPECIFICATION FOR ROUND TIMBER PILES.

9.2. ALL TIMBER PILING SHALL BE SOUTHERN PINE.

9.3. ALL TIMBER PILING SHALL CONFORM TO ASTM D25, CLASS A WITH A MINIMUM TIP DIAMETER OF 8" AND MINIMUM BUTT DIAMETER OF 14" THREE FEET FROM THE BUTT.

9.4. EACH PILE SHALL BE IDENTIFIED WITH A QUALITY MARK BY AN APPROVED INSPECTION AGENCY CERTIFIED BY THE AMERICAN LUMBER STANDARDS COMMITTEE (ALSC).

9.5. ALL TIMBER PILES SHALL BE TREATED IN ACCORDANCE WITH AMERICAN WOOD PROTECTION ASSOCIATION (AWPA) STANDARD UC4B TO 0.6 PCF CCA FOR HEAVY DUTY GROUND CONTACT FOUNDATION SYSTEMS.

9.6. PILING CONTRACTOR SHALL CUT OFF PILES CLEAN AND HORIZONTAL AT THE PILE ELEVATIONS SHOWN ON THE DRAWINGS.

9.7. PILING CONTRACTOR SHALL COMPACT SOIL AROUND PILING TO THE BOTTOM OF THE CAP ELEVATIONS SHOWN ON THE DRAWINGS.

9.8. JETTING SHALL NOT BE PERMITTED UNLESS APPROVED BY THE GEOTECHNICAL ENGINEER.

9.9. ALL PILES SHALL BE DRIVEN TO REFUSAL OR THE DEPTHS GIVEN IN THE GEOTECHNICAL REPORT. IF REFUSAL IS ENCOUNTERED, THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY. ANY OBSTRUCTION TO DRIVING TO THE SPECIFIED PENETRATION SHALL BE REMOVED.

9.10. PILES SHALL BE CAREFULLY LOCATED AS SHOWN IN THE PROJECT PLANS WITH A TOLERANCE OF 3 INCHES. PILING SHALL BE PLACED PLUMB WITH OUT-OF-PLUMBNESS NOT EXCEEDING 1/8 INCH PER FOOT.

9.11. PILINGS SHALL NOT BE DRIVEN WITHIN 100 FEET OF CONCRETE LESS THAN 7 DAYS OLD.

10. CONCRETE:

10.1. ALL CONCRETING OPERATIONS SHALL COMPLY WITH ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS".

10.2. DETAIL CONCRETE REINFORCEMENT AND ACCESSORIES IN ACCORDANCE WITH ACI 315 "DETAILING MANUAL".

10.3. THE CONTRACTOR SHALL SUBMIT FOR THE STRUCTURAL ENGINEER'S REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS. ITEMS MARKED (#) SHALL BE SUBMITTED FOR THE STRUCTURAL ENGINEER'S RECORD ONLY.

10.3.1. CONCRETE MIX DESIGNS

10.3.2. CONCRETE REINFORCING

10.3.3. CONSTRUCTION JOINT LOCATIONS IN STRUCTURAL FLOORS (#)

10.4. CONTRACTOR SHALL NOT FABRICATE OR PLACE REINFORCEMENT UNTIL REINFORCEMENT SHOP DRAWINGS, REVIEWED AND STAMPED BY THE STRUCTURAL ENGINEER, ARE RECEIVED ON THE JOB SITE. SHOP DRAWINGS SHALL CONSIST OF BOTH "CUT" AND PLACEMENT SHEETS. PLACEMENT SHEETS SHALL CONTAIN ALL INFORMATION NECESSARY TO POSITION ALL REINFORCING STEEL IN THE FIELD WITHOUT HAVING TO REFER TO THE STRUCTURAL DRAWINGS. ARCHITECTURAL AND STRUCTURAL DRAWINGS SHALL NOT BE COPIED OR REPRODUCED FOR USE AS SHOP DRAWINGS.

10.5. A QUALITY ASSURANCE PROGRAM CONSISTING OF SUBMITTALS, TESTING, AND INSPECTIONS SHALL BE USED TO VERIFY THAT CONSTRUCTION IS IN CONFORMANCE WITH THE CONTRACT DOCUMENTS. MATERIAL QUALITY, HANDLING, STORAGE, PREPARATION, PLACEMENT, AND CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE BUILDING CODE.

10.6. THE PROPOSED MATERIALS AND MIX DESIGN SHALL BE FULLY DOCUMENTED AND REVIEWED BY THE OWNER'S TESTING LABORATORY. RESPONSIBILITY FOR OBTAINING THE REQUIRED CONCRETE DESIGN STRENGTH IS THE CONTRACTOR'S.

10.7. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.

10.8. REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A706.

10.9. TIE ALL REINFORCING STEEL AND EMBEDMENT'S SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN POSITION OF REINFORCEMENT WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES. "STICKING" DOWELS INTO WET CONCRETE IS NOT PERMITTED.

10.10. WELDED WIRE REINFORCEMENT (WWR) SHALL CONFORM TO ASTM A1064. MINIMUM LAP AND EMBEDMENT TO BE THE GREATER OF ONE CROSS WIRE SPACING PLUS 2" OR 8". WWR SHALL BE SUPPLIED IN FLAT SHEETS (NOT ROLLS).

10.11. WELDED WIRE REINFORCEMENT SHALL BE SUFFICIENTLY SUPPORTED TO HOLD IT IN PLACE WITHIN ALLOWABLE TOLERANCES DURING THE PLACEMENT OF CONCRETE. WWR LOCATION SHALL NOT BE AFFECTED BY CONCRETE PLACEMENT, WORKER ACTIVITY, CONSOLIDATION, OR FINISHING.

10.12. DEFORMED BAR ANCHORS (DBA'S) SHALL CONFORM TO ASTM A496. DBA'S SHALL BE AUTOMATICALLY END WELDED USING MANUFACTURERS RECOMMENDED PROCEDURES, EQUIPMENT, FLUX, AND FERRULES. DBA'S SHALL BE NELSON FLUXED DBA'S OR APPROVED ALTERNATE.

10.13. SEE CONCRETE MIX DESIGN SCHEDULE FOR REQUIRED CONCRETE STRENGTH AND PROPERTIES.

10.14. SEE SECTIONS AND DETAILS FOR CONCRETE COVER. FOR CONCRETE COVERS NOT INDICATED IN SECTIONS AND DETAILS, SEE CONCRETE COVER SCHEDULE FOR REQUIRED STEEL COVERAGE.

10.15. THE USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED.

10.16. ALL EXPOSED CONCRETE EDGES SHALL HAVE A 3/4 INCH CHAMFER.

10.17. CONSTRUCTION JOINTS IN A HORIZONTAL PLANE ARE NOT PERMITTED.

10.18. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL BULKHEADS AND HORIZONTAL KEYS. MAKE ALL REINFORCING CONTINUOUS THROUGH CONSTRUCTION JOINTS. CONTROL JOINTS FOR CONCRETE SLABS ON GRADE SHALL BE AS DETAILED AND LOCATED AS SHOWN IN THE CONSTRUCTION DOCUMENTS.

10.19. EARTH SUPPORTED SLABS: 4" THICK, REINFORCED WITH 6X6 W2.9/W2.9 WWR AT MID-DEPTH OF SLAB, UNLESS NOTED.

10.20. COAT ALL SLABS WITH CURING COMPOUND WITHIN 24 HOURS OF PLACING. PRODUCT USED SHALL CONFORM WITH ASTM C309, AND SHALL BE COMPATIBLE WITH ADHERED FINISHES. A DISSIPATING FORMULATION SHALL BE USED AT CEMENTITIOUS FINISHES.

10.21. SLAB JOINTS SHALL BE FILLED WITH AN APPROVED MATERIAL. THIS SHOULD TAKE PLACE AS LATE AS POSSIBLE, PREFERABLY 4 TO 6 WEEKS AFTER THE SLAB HAS BEEN CAST. PRIOR TO FILLING, REMOVE ALL DEBRIS FROM THE SLAB JOINTS, THEN FILL IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS AS FOLLOWS: 6" SLABS FILL WITH EPOXY RESIN, OTHER SLABS FILL WITH FIELD MOLDED OR ELECTROMETRIC SEALANT.

10.22. SEE ARCHITECTURAL DRAWINGS FOR LOCATION OF DEPRESSED SLABS AND DRAINS. SLOPE SLAB TO DRAINS WHERE SHOWN.

10.23. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND VENDOR DRAWINGS FOR SLEEVES, EMBEDDED ITEMS, ACCESSORIES, ETC. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING AND PLACING ALL SLEEVES, EMBEDDED ITEMS, ACCESSORIES, ETC.

10.24. DO NOT PLACE PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS WITHIN THE SLAB UNLESS SPECIFICALLY SHOWN AND DETAILED ON THE STRUCTURAL DRAWINGS.

10.25. SEE SCHEDULES, SECTION NOTES, GENERAL NOTES, SECTIONS, AND DETAILS FOR ACTUAL REINFORCING REQUIRED.

10.26. REINFORCING BAR PLACING ACCESSORIES SHALL BE IN ACCORDANCE WITH ACI MANUAL OF STANDARD PRACTICE. WHERE CONCRETE IS EXPOSED IN FINISHED BUILDING, PROVIDE ACCESSORIES WITH RUSTPROOF LEGS. WHERE CONCRETE IS SAND-BLASTED OR BUSH-HAMMERED, PROVIDE ACCESSORIES OF STAINLESS STEEL.

10.27. ALL SPLICES SHALL BE CLASS "B" TENSION LAP SPLICE, UNLESS NOTED OTHERWISE.

10.28. WHERE REINFORCING BARS ARE NOTED AS CONTINUOUS, THE FOLLOWING REQUIREMENTS APPLY:

10.28.1. THE TERMINATION OF ALL CONTINUOUS REINFORCING BAR RUNS SHALL BE A STANDARD HOOK UNLESS NOTED OTHERWISE.

10.28.2. SPLICES IN CONTINUOUS TOP BARS SHALL OCCUR OVER PARALLEL WALLS OR AT THE CENTER OF THE CLEAR SPAN.

10.28.3. SPLICES IN CONTINUOUS BOTTOM BARS SHALL OCCUR OVER PERPENDICULAR WALLS OR CENTERED OVER COLUMNS.

10.29. HOOKS IN REINFORCING ARE IN ADDITION TO THE LENGTH SHOWN.

10.30. FIELD BENDING OF BARS LARGER THAN #4 IS NOT PERMITTED. ALL BENDS FOR BARS LARGER THAN #4 SHALL BE SHOP MADE COLD BENDS.

10.31. FOR PEDESTAL, COLUMN, AND WALL VERTICAL REINFORCING, DOWEL TO FOUNDATION WITH HOOKED BARS OF SAME SIZE AND SPACING AS VERTICAL REINFORCING.

10.32. FOR CONCRETE WALLS WITH A SINGLE LAYER OF REINFORCING, REINFORCING TO BE CENTERED IN WALL UNLESS NOTED.

10.33. ALL CONCRETE USED FOR WALL CONSTRUCTION SHALL CONTAIN THE FOLLOWING ADMIXTURES OR APPROVED EQUAL:

10.33.1. SPECIFICATION PRODUCTS E5 INTERNAL CURE

10.33.2. SPECIFICATION PRODUCTS E5 LIQUID FLY ASH

10.33.3. CORTEC MCI-2005 NS

11. STRUCTURAL STEEL:

11.1. FABRICATE AND ERECT ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".

11.2. THE CONTRACTOR SHALL SUBMIT FOR THE STRUCTURAL ENGINEER'S REVIEW SHOP DRAWINGS WHICH INCLUDE ERECTION DRAWINGS, MATERIALS, CONNECTIONS, FABRICATION, AND ALL DETAILS FOR THE FOLLOWING ITEMS.

11.2.1. STRUCTURAL STEEL

11.3. THE STEEL FRAME IS "NON-SELF-SUPPORTING". ADEQUATE TEMPORARY SUPPORT MUST BE PROVIDED BY THE CONTRACTOR UNTIL THE LATERAL LOAD RESISTANCE SYSTEM IS INSTALLED AND STABILITY OF THE COMPLETED STRUCTURE IS ACHIEVED. THE LATERAL LOAD RESISTANCE SYSTEM AND STABILITY OF THE STRUCTURE IS PROVIDED BELOW:

11.3.1. ROOF DIAPHRAGM: STEEL PURLINS

11.3.2. FLOOR DIAPHRAGM: COMPOSITE SLAB ON DECK

11.3.3. COLLECTOR ELEMENTS/DRAG STRUTS: WIDE FLANGE BEAMS

11.3.4. LATERAL LOAD RESISTING SYSTEM: ORDINARY MOMENT FRAME

11.4. STRUCTURAL STEEL:

11.4.1. ASTM 572, GR 50 FOR ALL EMBED PLATE STEEL

11.4.2. ASTM A572, GR 50 FOR STEEL ANGLES

11.4.3. ASTM A847 FOR CORTEN HOLLOW STRUCTURAL SECTIONS

11.5. WELDED CONNECTIONS: E7018-W1 OR E7018-W2 ELECTRODES, MINIMUM SIZE FILLET WELD 3/16". ALL SHOP AND FIELD WELDING SHALL BE BY A CERTIFIED WELDER AND IN ACCORDANCE WITH AMERICAN WELDING SOCIETY D1.1 SPECIFICATION.

11.6. ENGINEER SHALL BE CONTACTED FOR APPROVAL OF ANY FIELD MODIFICATIONS OR REPAIRS OF ANCHOR BOLTS OR RODS, AND COLUMN BASE PLATES.

11.7. SHEAR CONNECTORS: ASTM A108, GRADE 1015 THROUGH 1020, HEADED-STUD TYPE, COLD FINISHED CARBON STEEL; AWS D1.1, TYPE B.

11.8. ALL STEEL ELEMENTS EXCEPT FOR CORTEN SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123 AFTER SANDBLAST CLEANING PER SSPC-SP10. USE ASTM A325 BOLTS HOT DIPPED GALVANIZED WITH GALVANIZED HARDENED WASHERS AND GALVANIZED HEAVY HEX NUTS FOR BOLTING OF GALVANIZED ITEMS.

11.9. STEEL COLUMNS, BASE PLATES AND ALL STEEL BELOW GRADE SHALL HAVE A MINIMUM 3" CONCRETE COVER.

12. WOOD FRAMING:

12.1. WOOD CONSTRUCTION SHALL COMPLY WITH THE INTERNATIONAL BUILDING CODE AND THE AMERICAN WOOD COUNCIL REQUIREMENTS.

12.2. A QUALITY ASSURANCE PROGRAM CONSISTING OF SUBMITTALS AND INSPECTIONS SHALL BE USED TO VERIFY THAT THE CONSTRUCTED WOOD IS IN CONFORMANCE WITH THE CONTRACT DOCUMENTS. MATERIAL QUALITY, HANDLING, STORAGE, PREPARATION, PLACEMENT, AND CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE CODE.

12.3. WOOD FRAMING MEMBERS: VISUALLY GRADED DIMENSIONED #2 SOUTHERN PINE.

12.4. PRESERVATIVE RETENTION:

12.4.1. 0.60 LBS/FT3 PERMANENT WOOD FOUNDATIONS

12.4.2. 0.40 LBS/FT3 GROUND CONTACT

12.4.3. 0.25 LBS/FT3 ABOVE GROUND

12.5. ALL FASTENERS, NAILS AND OTHER METAL PRODUCTS USED WITH PRESSURE TREATED LUMBER SHALL BE HOT-DIP GALVANIZED, STAINLESS STEEL, OR AS RECOMMENDED BY THE PRESERVATIVE MANUFACTURER. PRESSURE TREATED LUMBER SHALL NOT BE IN DIRECT CONTACT WITH ALUMINUM PRODUCTS.

12.6. ALL MANUFACTURED WOOD FRAMING CONNECTORS TO BE BY SIMPSON STRONG-TIE COMPANY, INC. OR APPROVED EQUAL. ALL CONNECTORS SHALL BE FASTENED TO FRAMING MEMBERS FILLING THE REQUIRED NUMBER OF CONNECTOR HOLES WITH THE TYPE AND SIZE FASTENERS SPECIFIED BY THE MANUFACTURER.

12.7. ROOF T&G SHEATHING SCREWS, UNLESS NOTED: SIMPSON STRONG-TIE COMPANY, INC. STRONG-DRIVE TF WOOD-TO-STEEL SCREW (MODEL NO. TFP1475) WITH A MINIMUM OF 2 FASTENERS AT EACH PURLIN SUPPORT.

13. POST INSTALLED ANCHORS:

13.1. POST INSTALLED ANCHORS SHALL COMPLY WITH ACI-318.

13.2. POST INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE GENERAL CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USING POST INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.

13.3. ACCEPTABLE MANUFACTURERS SHALL INCLUDE BUT ARE NOT LIMITED TO HILTI, INC. AND SIMPSON STRONG-TIE COMPANY, INC.

13.4. CARE SHALL BE TAKEN IN PLACING POST INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR.

13.5. HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS. SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SHOWN SHALL BE SUBMITTED BY THE CONTRACTOR ALONG WITH PREPARED DOCUMENTATION DEMONSTRATING EQUAL SUBSTITUTION THAT THE PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE.

13.6. THE CONTRACTOR SHALL FOLLOW ALL MANUFACTURER'S INSTALLATION GUIDELINES, SPECIFICATIONS, AND RECOMMENDATIONS.

13.7. A REPRESENTATIVE OF THE POST-INSTALLED ANCHOR MANUFACTURER SHALL BE PRESENT FOR THE FIRST INSTALLATION OF EACH TYPE OF ANCHOR USED TO DEMONSTRATE AND INSTRUCT TO THE CONTRACTOR'S INSTALLATION CREW AND PERSONNEL THE PROPER METHOD OF INSTALLATION. SHOULD THE CONTRACTOR CHANGE INSTALLATION CREW OR INDIVIDUALS INSTALLING THE ANCHOR, THE MANUFACTURER'S REPRESENTATIVE SHALL BE NOTIFIED BY THE CONTRACTOR TO RETURN AND PROVIDE INSTRUCTION TO THE NEW INSTALLER(S).

13.8. MECHANICAL ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI-355.2 AND ICC-ES AC193.

13.9. ADHESIVE ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI355.4 AND ICC-ES AC308.

13.10. ADHESIVE ANCHORS WITH SCREEN TUBES SHALL BE TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC58 OR AC60, AS APPROPRIATE. THE APPROPRIATE SCREEN TUBE SHALL BE USED AS RECOMMENDED BY THE ADHESIVE MANUFACTURER.

14. FOUNDATION QUALITY CONTROL:

14.1. BEARING ELEVATIONS: THE TOP ELEVATION OF ALL FOOTINGS IS SHOWN ON THE DRAWINGS FOR BID PURPOSES. THE FINAL BEARING ELEVATIONS MAY VARY AS REQUIRED TO PROVIDE PROPER BEARING CAPACITY IN AN APPROVED BEARING STRATUM AS DETERMINED BY THE GEOTECHNICAL ENGINEER.

14.2. FIELD INSPECTION OF BEARING STRATUM: THE BEARING STRATUM OF EACH SPREAD FOOTING SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO POURING OF CONCRETE.

14.3. FOOTINGS SHALL BE NEATLY EXCAVATED WHERE POSSIBLE WITH SIDES AND TOP EDGES FREE OF LOOSE OR WET MATERIALS. WHERE NEAT EXCAVATION IS NOT POSSIBLE, FOOTING EXCAVATION SHALL BE OPEN CUT WITH EDGES FORMED AND BRACED. ALL FOOTINGS WITH FORMED EDGES SHALL BE BACKFILLED FROM BOTTOM TO TOP OF FOOTING WITH LEAN CONCRETE. THE BOTTOM EXCAVATION SHALL BE CLEAN AND DRY WITH ALL LOOSE MATERIAL REMOVED FOR AN ESSENTIALLY FLAT BEARING SURFACE. EXCAVATIONS SHALL NOT BE LEFT OVERNIGHT UNLESS A 2" UNREINFORCED CONCRETE SEAL (MUD) SLAB IS PLACED AT THE BOTTOM OF THE FOOTING EXCAVATION. WHERE SOFT OR UNSUITABLE BEARING SURFACES ARE ENCOUNTERED, THE AREA SHALL BE UNDERCUT AS REQUIRED AND REPLACED WITH LEAN CONCRETE OR COMPACTED DENSE GRADED CRUSHED STONE AS DIRECTED BY THE GEOTECHNICAL ENGINEER.

14.4. ALL BACKFILL SHALL BE ENGINEERED FILL AS DEFINED IN THE GEOTECHNICAL REPORT. EXCAVATED MATERIAL MAY BE USED AS BACKFILL MATERIAL WITH WRITTEN APPROVAL FROM THE GEOTECHNICAL ENGINEER STATING THAT SUCH MATERIAL IS SUITABLE AS BACKFILL AND INSTRUCTIONS ARE GIVEN FOR PROPER MOISTURE CONTENT AND COMPACTION. THE TESTING AGENCY APPROVAL AND INSTRUCTIONS FOR COMPACTION SHALL BE SUBMITTED TO THE GEOTECHNICAL ENGINEER FOR REVIEW.

15. CONCRETE QUALITY CONTROL:

15.1. ALL CONCRETE TO BE AIR ENTRAINED SHALL USE AIR-ENTRAINING ADMIXTURE AT THE MANUFACTURER'S PRESCRIBED RATE TO RESULT IN CONCRETE AT THE POINT OF PLACEMENT HAVING A TOTAL AIR CONTENT AS NOTED ABOVE.

15.2. CONCRETE AGGREGATES SHALL CONFORM TO ASTM C33. NORMAL WEIGHT CONCRETE AGGREGATES MAY BE EITHER GRAVEL OR LIMESTONE UNLESS SPECIFIED.

15.3. WATER FOR CONCRETE SHALL BE CLEAN, FRESH, AND DRINKABLE.

15.4. CEMENT SHALL CONFORM TO THE SPECIFICATION FOR PORTLAND CEMENT, ASTM C150, TYPE I (NORMAL).

15.5. UNLESS ACCEPTED BY THE STRUCTURAL ENGINEER, USE ONE BRAND OF CEMENT THROUGHOUT THE PROJECT.

15.6. AN INDEPENDENT TESTING AGENCY SHALL PREPARE DESIGN MIXES FOR EACH TYPE AND STRENGTH OF CONCRETE BY EITHER LABORATORY TRIAL MIXTURES OR FIELD EXPERIENCE METHODS AS SPECIFIED IN ACI 318.

15.7. CONCRETE MIX DESIGNS MUST BE SUBMITTED A MINIMUM OF 15 DAYS PRIOR TO THE START OF THE WORK FOR STRUCTURAL ENGINEER'S ACCEPTANCE. ANY ADJUSTMENT IN APPROVED MIX DESIGNS INCLUDING CHANGES IN ADMIXTURES MUST BE SUBMITTED IN WRITING TO THE STRUCTURAL ENGINEER FOR ACCEPTANCE PRIOR TO USE IN THE FIELD.

15.8. CONCRETE DESIGNED TO BE PUMPED SHALL BE SO NOTED ON THE MIX DESIGNS AND SHALL HAVE MIX PROPORTIONS COMPATIBLE WITH THE PUMPING PROCESS.

15.9. USE ONLY ADMIXTURES APPROVED BY THE STRUCTURAL ENGINEER AND CONTAINING NO CHLORIDE IONS.

15.10. THE CONTRACTOR SHALL EMPLOY A TESTING AGENCY TO PERFORM THE REQUIRED TESTS AND TO SUBMIT THE TEST REPORTS.

15.11. DURING PLACEMENT OF CONCRETE SAMPLE AND TEST CONCRETE FOR QUALITY CONTROL AS FOLLOWS:

15.11.1. CONCRETE SAMPLING: ASTM C172, EXCEPT MODIFIED FOR SLUMP TO COMPLY WITH ASTM C94.

15.11.2. CONCRETE SLUMP: ASTM C143, ONE TEST FOR EACH SET OF COMPRESSIVE STRENGTH TEST SPECIMENS.

15.11.3. AIR CONTENT: ASTM C173, VOLUMETRIC METHOD FOR LIGHTWEIGHT OR NORMAL WEIGHT CONCRETE; ASTM C231 PRESSURE FOR NORMAL WEIGHT CONCRETE; ONE FOR EACH SET OF COMPRESSIVE STRENGTH TEST SPECIMENS.

15.11.4. CONCRETE TEMPERATURE: TEST HOURLY WHEN AIR TEMPERATURE IS 40 DEGREES F (4 DEGREES C) AND BELOW, AND WHEN 80 DEGREES F (27 DEGREES C) AND ABOVE, AND EACH TIME A SET OF COMPRESSION TEST SPECIMENS ARE MADE.

15.11.5. COMPRESSIVE TEST SPECIMEN: ASTM C31, ONE SET OF FOUR STANDARD CYLINDERS FOR EACH COMPRESSIVE STRENGTH TEST, UNLESS DIRECTED OTHERWISE. MOLD AND STORE CYLINDERS FOR LABORATORY CURED TEST SPECIMENS EXCEPT WHEN FIELD-CURE TEST SPECIMENS ARE REQUIRED.

15.11.6. COMPRESSIVE STRENGTH TESTS: ASTM C39, ONE SET FOR EACH 50 CUBIC YARDS OR FRACTION THEREOF, OF EACH CONCRETE CLASS PLACED IN ANY ONE DAY OR FOR EACH 5,000 SQ. FT. OF SURFACE AREA PLACED. TEST ONE SPECIMEN AT 7 DAYS, TWO SPECIMENS AT 28 DAYS, AND RETAIN ONE SPECIMEN IN RESERVE FOR LATER TESTING IF REQUIRED.

15.11.7. WHEN FREQUENCY OF TESTING WILL PROVIDE LESS THAN 5 STRENGTH TESTS FOR A GIVEN CLASS OF CONCRETE, CONDUCT TESTING FROM AT LEAST 5 RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN 5 ARE USED.

15.11.8. WHEN STRENGTH OF FIELD-CURED CYLINDERS IS LESS THAN 85 PERCENT OF COMPANION LABORATORY-CURED CYLINDERS, EVALUATE CURRENT OPERATIONS AND PROVIDE CORRECTIVE PROCEDURES FOR PROTECTING AND CURING THE IN-PLACE CONCRETE.

15.11.9. STRENGTH LEVEL OF CONCRETE WILL BE CONSIDERED SATISFACTORY IF AVERAGES OF SETS OF THREE CONSECUTIVE STRENGTH TEST RESULTS EQUAL OR EXCEED SPECIFIED COMPRESSIVE STRENGTH, AND NO INDIVIDUAL STRENGTH TEST RESULT FALLS BELOW SPECIFIED COMPRESSIVE STRENGTH BY MORE THAN 500 PSI.

15.12. TEST RESULTS WILL BE REPORTED IN WRITING TO THE OWNER, ARCHITECT, STRUCTURAL ENGINEER, AND CONTRACTOR. REPORTS OF COMPRESSIVE STRENGTH TESTS SHALL CONTAIN THE PROJECT IDENTIFICATION NAME AND NUMBER, DATE OF CONCRETE PLACEMENT, NAME OF CONCRETE TESTING AGENCY, CONCRETE TYPE AND CLASS, LOCATION OF CONCRETE BATCH IN STRUCTURE, DESIGN COMPRESSIVE STRENGTH AT 28 DAYS, CONCRETE MIX PROPORTIONS AND MATERIAL; COMPRESSIVE BREAKING STRENGTH AND TYPE OF BREAK FOR BOTH 7-DAY TESTS AND 28-DAY TESTS.

15.13. NONDESTRUCTIVE TESTING: IMPACT HAMMER, SONOSCOPE, OR OTHER NONDESTRUCTIVE DEVICE MAY BE PERMITTED BUT SHALL NOT BE USED AS THE SOLE BASIS FOR ACCEPTANCE OR REJECTION.

15.14. ADDITIONAL TESTS: THE TESTING AGENCY SHALL MAKE ADDITIONAL TESTS OF IN-PLACE CONCRETE WHEN TEST RESULTS INDICATE SPECIFIED CONCRETE STRENGTHS AND OTHER CHARACTERISTICS HAVE NOT BEEN ATTAINED IN THE STRUCTURE, AS DIRECTED BY THE STRUCTURAL ENGINEER. CONTRACTOR SHALL PAY FOR SUCH TESTS CONDUCTED AND ANY OTHER ADDITIONAL TESTING AS MAY BE REQUIRED WHEN UNACCEPTABLE CONCRETE IS VERIFIED.

16. STEEL QUALITY CONTROL:

16.1. FIELD INSPECTIONS AND TESTS: CHECK STEEL AS RECEIVED IN THE FIELD FOR POSSIBLE SHIPPING DAMAGE, WORKMANSHIP, PIECE MARKING, AND REQUIRED CAMBER.

16.2. SHOP AND FIELD WELDING INSPECTION AND TESTING DURING FABRICATION AND ERECTION OF STRUCTURAL STEEL IN ACCORDANCE WITH AISC SPECIFICATIONS AND AS

REVISION NO.	DESCRIPTION	DATE	BY:
0	ISSUE FOR BID	2/23/24	JHB

ALABAMA LICENSED No. 50027 PROFESSIONAL 02/23/2024 ENGINEER JARED H. BLOUNT

CITY OF FAIRHOPE
FAIRHOPE, ALABAMA

FLYING CREEK NATURE PRESERVE

thompson ENGINEERING
2970 COTTAGE HILL RD., STE. 190
MOBILE, ALABAMA 36606
TEL: (251) 666-2443
FAX: (251) 666-6422

GENERAL NOTES

SCALE:	PLOT SCALE:	DRAWN BY:	CHECKED BY:	APPROVED BY:	DATE :	JOB NO. :	DRAWING NO.	REVISION NO. :
NOTED	1:1	TPT	RAH	JHB	FEBRUARY 2024	22-1101-0229	S001	0

10.10. WELDED WIRE REINFORCEMENT (WWR) SHALL CONFORM TO ASTM A1064. MINIMUM LAP AND EMBEDMENT TO BE THE GREATER OF ONE CROSS WIRE SPACING PLUS 2" OR 8". WWR SHALL BE SUPPLIED IN FLAT SHEETS (NOT ROLLS).

10.11. WELDED WIRE REINFORCEMENT SHALL BE SUFFICIENTLY SUPPORTED TO HOLD IT IN PLACE WITHIN ALLOWABLE TOLERANCES DURING THE PLACEMENT OF CONCRETE. WWR LOCATION SHALL NOT BE AFFECTED BY CONCRETE PLACEMENT, WORKER ACTIVITY, CONSOLIDATION, OR FINISHING.

10.12. DEFORMED BAR ANCHORS (DBA'S) SHALL CONFORM TO ASTM A496. DBA'S SHALL BE AUTOMATICALLY END WELDED USING MANUFACTURERS RECOMMENDED PROCEDURES, EQUIPMENT, FLUX, AND FERRULES. DBA'S SHALL BE NELSON FLUXED DBA'S OR APPROVED ALTERNATE.

10.13. SEE CONCRETE MIX DESIGN SCHEDULE FOR REQUIRED CONCRETE STRENGTH AND PROPERTIES.

10.14. SEE SECTIONS AND DETAILS FOR CONCRETE COVER. FOR CONCRETE COVERS NOT INDICATED IN SECTIONS AND DETAILS, SEE CONCRETE COVER SCHEDULE FOR REQUIRED STEEL COVERAGE.

10.15. THE USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED.

10.16. ALL EXPOSED CONCRETE EDGES SHALL HAVE A 3/4 INCH CHAMFER.

10.17. CONSTRUCTION JOINTS IN A HORIZONTAL PLANE ARE NOT PERMITTED.

10.18. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL BULKHEADS AND HORIZONTAL KEYS. MAKE ALL REINFORCING CONTINUOUS THROUGH CONSTRUCTION JOINTS. CONTROL JOINTS FOR CONCRETE SLABS ON GRADE SHALL BE AS DETAILED AND LOCATED AS SHOWN IN THE CONSTRUCTION DOCUMENTS.

10.19. EARTH SUPPORTED SLABS: 4" THICK, REINFORCED WITH 6X6 W2.9/W2.9 WWR AT MID-DEPTH OF SLAB, UNLESS NOTED.

10.20. COAT ALL SLABS WITH CURING COMPOUND WITHIN 24 HOURS OF PLACING. PRODUCT USED SHALL CONFORM WITH ASTM C309, AND SHALL BE COMPATIBLE WITH ADHERED FINISHES. A DISSIPATING FORMULATION SHALL BE USED AT CEMENTITIOUS FINISHES.

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11.6. ENGINEER SHALL BE CONTACTED FOR APPROVAL OF ANY FIELD MODIFICATIONS OR REPAIRS OF ANCHOR BOLTS OR RODS, AND COLUMN BASE PLATES.

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11.9. STEEL COLUMNS, BASE PLATES AND ALL STEEL BELOW GRADE SHALL HAVE A MINIMUM 3" CONCRETE COVER.

12. WOOD FRAMING:

12.1. WOOD CONSTRUCTION SHALL COMPLY WITH THE INTERNATIONAL BUILDING CODE AND THE AMERICAN WOOD COUNCIL REQUIREMENTS.

12.2. A QUALITY ASSURANCE PROGRAM CONSISTING OF SUBMITTALS AND INSPECTIONS SHALL BE USED TO VERIFY THAT THE CONSTRUCTED WOOD IS IN CONFORMANCE WITH THE CONTRACT DOCUMENTS. MATERIAL QUALITY, HANDLING, STORAGE, PREPARATION, PLACEMENT, AND CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE CODE.

12.3. WOOD FRAMING MEMBERS: VISUALLY GRADED DIMENSIONED #2 SOUTHERN PINE.

12.4. PRESERVATIVE RETENTION:

12.4.1.0.60 LBS/FT3 PERMANENT WOOD FOUNDATIONS

12.4.2.0.40 LBS/FT3 GROUND CONTACT

12.4.3.0.25 LBS/FT3 ABOVE GROUND

12.5. ALL FASTENERS, NAILS AND OTHER METAL PRODUCTS USED WITH PRESSURE TREATED LUMBER SHALL BE HOT-DIP GALVANIZED, STAINLESS STEEL, OR AS RECOMMENDED BY THE PRESERVATIVE MANUFACTURER. PRESSURE TREATED LUMBER SHALL NOT BE IN DIRECT CONTACT WITH ALUMINUM PRODUCTS.

12.6. ALL MANUFACTURED WOOD FRAMING CONNECTORS TO BE BY SIMPSON STRONG-TIE COMPANY, INC. OR APPROVED EQUAL. ALL CONNECTORS SHALL BE FASTENED TO FRAMING MEMBERS FILLING THE REQUIRED NUMBER OF CONNECTOR HOLES WITH THE TYPE AND SIZE FASTENERS SPECIFIED BY THE MANUFACTURER.

12.7. ROOF T&G SHEATHING SCREWS, UNLESS NOTED: SIMPSON STRONG-TIE COMPANY, INC. STRONG-DRIVE TF WOOD-TO-STEEL SCREW (MODEL NO. TFP1475) WITH A MINIMUM OF 2 FASTENERS AT EACH PURLIN SUPPORT.

13. POST INSTALLED ANCHORS:

13.1. POST INSTALLED ANCHORS SHALL COMPLY WITH ACI-318.

13.2. POST INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE GENERAL CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USING POST INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.

13.3. ACCEPTABLE MANUFACTURERS SHALL INCLUDE BUT ARE NOT LIMITED TO HILTI, INC. AND SIMPSON STRONG-TIE COMPANY, INC.

13.4. CARE SHALL BE TAKEN IN PLACING POST INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR.

13.5. HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS. SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SHOWN SHALL BE SUBMITTED BY THE CONTRACTOR ALONG WITH PREPARED DOCUMENTATION DEMONSTRATING EQUAL SUBSTITUTION THAT THE PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE.

13.6. THE CONTRACTOR SHALL FOLLOW ALL MANUFACTURER'S INSTALLATION GUIDELINES, SPECIFICATIONS, AND RECOMMENDATIONS.

13.7. A REPRESENTATIVE OF THE POST-INSTALLED ANCHOR MANUFACTURER SHALL BE PRESENT FOR THE FIRST INSTALLATION OF EACH TYPE OF ANCHOR USED TO DEMONSTRATE AND INSTRUCT TO THE CONTRACTOR'S INSTALLATION CREW AND PERSONNEL THE PROPER METHOD OF INSTALLATION. SHOULD THE CONTRACTOR CHANGE INSTALLATION CREW OR INDIVIDUALS INSTALLING THE ANCHOR, THE MANUFACTURER'S REPRESENTATIVE SHALL BE NOTIFIED BY THE CONTRACTOR TO RETURN AND PROVIDE INSTRUCTION TO THE NEW INSTALLER(S).

13.8. MECHANICAL ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI-355.2 AND ICC-ES AC193.

13.9. ADHESIVE ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI355.4 AND ICC-ES AC308.

13.10. ADHESIVE ANCHORS WITH SCREEN TUBES SHALL BE TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC58 OR AC60, AS APPROPRIATE. THE APPROPRIATE SCREEN TUBE SHALL BE USED AS RECOMMENDED BY THE ADHESIVE MANUFACTURER.

14. FOUNDATION QUALITY CONTROL:

14.1. BEARING ELEVATIONS: THE TOP ELEVATION OF ALL FOOTINGS IS SHOWN ON THE DRAWINGS FOR BID PURPOSES. THE FINAL BEARING ELEVATIONS MAY VARY AS REQUIRED TO PROVIDE PROPER BEARING CAPACITY IN AN APPROVED BEARING STRATUM AS DETERMINED BY THE GEOTECHNICAL ENGINEER.

14.2. FIELD INSPECTION OF BEARING STRATUM: THE BEARING STRATUM OF EACH SPREAD FOOTING SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO POURING OF CONCRETE.

14.3. FOOTINGS SHALL BE NEATLY EXCAVATED WHERE POSSIBLE WITH SIDES AND TOP EDGES FREE OF LOOSE OR WET MATERIALS. WHERE NEAT EXCAVATION IS NOT POSSIBLE, FOOTING EXCAVATION SHALL BE OPEN CUT WITH EDGES FORMED AND BRACED. ALL FOOTINGS WITH FORMED EDGES SHALL BE BACKFILLED FROM BOTTOM TO TOP OF FOOTING WITH LEAN CONCRETE. THE BOTTOM EXCAVATION SHALL BE CLEAN AND DRY WITH ALL LOOSE MATERIAL REMOVED FOR AN ESSENTIALLY FLAT BEARING SURFACE. EXCAVATIONS SHALL NOT BE LEFT OVERNIGHT UNLESS A 2" UNREINFORCED CONCRETE SEAL (MUD) SLAB IS PLACED AT THE BOTTOM OF THE FOOTING EXCAVATION. WHERE SOFT OR UNSUITABLE BEARING SURFACES ARE ENCOUNTERED, THE AREA SHALL BE UNDERCUT AS REQUIRED AND REPLACED WITH LEAN CONCRETE OR COMPACTED DENSE GRADED CRUSHED STONE AS DIRECTED BY THE GEOTECHNICAL ENGINEER.

14.4. ALL BACKFILL SHALL BE ENGINEERED FILL AS DEFINED IN THE GEOTECHNICAL REPORT. EXCAVATED MATERIAL MAY BE USED AS BACKFILL MATERIAL WITH WRITTEN APPROVAL FROM THE GEOTECHNICAL ENGINEER STATING THAT SUCH MATERIAL IS SUITABLE AS BACKFILL AND INSTRUCTIONS ARE GIVEN FOR PROPER MOISTURE CONTENT AND COMPACTION. THE TESTING AGENCY APPROVAL AND INSTRUCTIONS FOR COMPACTION SHALL BE SUBMITTED TO THE GEOTECHNICAL ENGINEER FOR REVIEW.

15. CONCRETE QUALITY CONTROL:

15.1. ALL CONCRETE TO BE AIR ENTRAINED SHALL USE AIR-ENTRAINING ADMIXTURE AT THE MANUFACTURER'S PRESCRIBED RATE TO RESULT IN CONCRETE AT THE POINT OF PLACEMENT HAVING A TOTAL AIR CONTENT AS NOTED ABOVE.

15.2. CONCRETE AGGREGATES SHALL CONFORM TO ASTM C33. NORMAL WEIGHT CONCRETE AGGREGATES MAY BE EITHER GRAVEL OR LIMESTONE UNLESS SPECIFIED.

15.3. WATER FOR CONCRETE SHALL BE CLEAN, FRESH, AND DRINKABLE.

15.4. CEMENT SHALL CONFORM TO THE SPECIFICATION FOR PORTLAND CEMENT, ASTM C150, TYPE I (NORMAL).

15.5. UNLESS ACCEPTED BY THE STRUCTURAL ENGINEER, USE ONE BRAND OF CEMENT THROUGHOUT THE PROJECT.

15.6. AN INDEPENDENT TESTING AGENCY SHALL PREPARE DESIGN MIXES FOR EACH TYPE AND STRENGTH OF CONCRETE BY EITHER LABORATORY TRIAL MIXTURES OR FIELD EXPERIENCE METHODS AS SPECIFIED IN ACI 318.

15.7. CONCRETE MIX DESIGNS MUST BE SUBMITTED A MINIMUM OF 15 DAYS PRIOR TO THE START OF THE WORK FOR STRUCTURAL ENGINEER'S ACCEPTANCE. ANY ADJUSTMENT IN APPROVED MIX DESIGNS INCLUDING CHANGES IN ADMIXTURES MUST BE SUBMITTED IN WRITING TO THE STRUCTURAL ENGINEER FOR ACCEPTANCE PRIOR TO USE IN THE FIELD.

15.8. CONCRETE DESIGNED TO BE PUMPED SHALL BE SO NOTED ON THE MIX DESIGNS AND SHALL HAVE MIX PROPORTIONS COMPATIBLE WITH THE PUMPING PROCESS.

15.9. USE ONLY ADMIXTURES APPROVED BY THE STRUCTURAL ENGINEER AND CONTAINING NO CHLORIDE IONS.

15.10. THE CONTRACTOR SHALL EMPLOY A TESTING AGENCY TO PERFORM THE REQUIRED TESTS AND TO SUBMIT THE TEST REPORTS.

15.11. DURING PLACEMENT OF CONCRETE SAMPLE AND TEST CONCRETE FOR QUALITY CONTROL AS FOLLOWS:

15.11.1. CONCRETE SAMPLING: ASTM C172, EXCEPT MODIFIED FOR SLUMP TO COMPLY WITH ASTM C94.

15.11.2. CONCRETE SLUMP: ASTM C143, ONE TEST FOR EACH SET OF COMPRESSIVE STRENGTH TEST SPECIMENS.

15.11.3. AIR CONTENT: ASTM C173, VOLUMETRIC METHOD FOR LIGHTWEIGHT OR NORMAL WEIGHT CONCRETE; ASTM C231 PRESSURE FOR NORMAL WEIGHT CONCRETE; ONE FOR EACH SET OF COMPRESSIVE STRENGTH TEST SPECIMENS.

15.11.4. CONCRETE TEMPERATURE: TEST HOURLY WHEN AIR TEMPERATURE IS 40 DEGREES F (4 DEGREES C) AND BELOW, AND WHEN 80 DEGREES F (27 DEGREES C) AND ABOVE, AND EACH TIME A SET OF COMPRESSION TEST SPECIMENS ARE MADE.

15.11.5. COMPRESSIVE TEST SPECIMEN: ASTM C31, ONE SET OF FOUR STANDARD CYLINDERS FOR EACH COMPRESSIVE STRENGTH TEST, UNLESS DIRECTED OTHERWISE. MOLD AND STORE CYLINDERS FOR LABORATORY CURED TEST SPECIMENS EXCEPT WHEN FIELD-CURE TEST SPECIMENS ARE REQUIRED.

15.11.6. COMPRESSIVE STRENGTH TESTS: ASTM C39, ONE SET FOR EACH 50 CUBIC YARDS OR FRACTION THEREOF, OF EACH CONCRETE CLASS PLACED IN ANY ONE DAY OR FOR EACH 5,000 SQ. FT. OF SURFACE AREA PLACED. TEST ONE SPECIMEN AT 7 DAYS, TWO SPECIMENS AT 28 DAYS, AND RETAIN ONE SPECIMEN IN RESERVE FOR LATER TESTING IF REQUIRED.

15.11.7. WHEN FREQUENCY OF TESTING WILL PROVIDE LESS THAN 5 STRENGTH TESTS FOR A GIVEN CLASS OF CONCRETE, CONDUCT TESTING FROM AT LEAST 5 RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN 5 ARE USED.

15.11.8. WHEN STRENGTH OF FIELD-CURED CYLINDERS IS LESS THAN 85 PERCENT OF COMPANION LABORATORY-CURED CYLINDERS, EVALUATE CURRENT OPERATIONS AND PROVIDE CORRECTIVE PROCEDURES FOR PROTECTING AND CURING THE IN-PLACE CONCRETE.

15.11.9. STRENGTH LEVEL OF CONCRETE WILL BE CONSIDERED SATISFACTORY IF AVERAGES OF SETS OF THREE CONSECUTIVE STRENGTH TEST RESULTS EQUAL OR EXCEED SPECIFIED COMPRESSIVE STRENGTH, AND NO INDIVIDUAL STRENGTH TEST RESULT FALLS BELOW SPECIFIED COMPRESSIVE STRENGTH BY MORE THAN 500 PSI.

15.12. TEST RESULTS WILL BE REPORTED IN WRITING TO THE OWNER, ARCHITECT, STRUCTURAL ENGINEER, AND CONTRACTOR. REPORTS OF COMPRESSIVE STRENGTH TESTS SHALL CONTAIN THE PROJECT IDENTIFICATION NAME AND NUMBER, DATE OF CONCRETE PLACEMENT, NAME OF CONCRETE TESTING AGENCY, CONCRETE TYPE AND CLASS, LOCATION OF CONCRETE BATCH IN STRUCTURE, DESIGN COMPRESSIVE STRENGTH AT 28 DAYS, CONCRETE MIX PROPORTIONS AND MATERIAL; COMPRESSIVE BREAKING STRENGTH AND TYPE OF BREAK FOR BOTH 7-DAY TESTS AND 28-DAY TESTS.

15.13. NONDESTRUCTIVE TESTING: IMPACT HAMMER, SONOSCOPE, OR OTHER NONDESTRUCTIVE DEVICE MAY BE PERMITTED BUT SHALL NOT BE USED AS THE SOLE BASIS FOR ACCEPTANCE OR REJECTION.

15.14. ADDITIONAL TESTS: THE TESTING AGENCY SHALL MAKE ADDITIONAL TESTS OF IN-PLACE CONCRETE WHEN TEST RESULTS INDICATE SPECIFIED CONCRETE STRENGTHS AND OTHER CHARACTERISTICS HAVE NOT BEEN ATTAINED IN THE STRUCTURE, AS DIRECTED BY THE STRUCTURAL ENGINEER. CONTRACTOR SHALL PAY FOR SUCH TESTS CONDUCTED AND ANY OTHER ADDITIONAL TESTING AS MAY BE REQUIRED WHEN UNACCEPTABLE CONCRETE IS VERIFIED.

16. STEEL QUALITY CONTROL:

16.1. FIELD INSPECTIONS AND TESTS: CHECK STEEL AS RECEIVED IN THE FIELD FOR POSSIBLE SHIPPING DAMAGE, WORKMANSHIP, PIECE MARKING, AND REQUIRED CAMBER.

16.2. SHOP AND FIELD WELDING INSPECTION AND TESTING DURING FABRICATION AND ERECTION OF STRUCTURAL STEEL IN ACCORDANCE WITH AISC SPECIFICATIONS AND AS FOLLOWS:

16.2.1. CERTIFY WELDERS AND CONDUCT INSPECTIONS AND TESTS AS REQUIRED.

16.2.2. RECORD TYPES AND LOCATIONS OF DEFECTS FOUND IN WORK.

16.2.3. RECORD WORK REQUIRED AND PERFORMED TO CORRECT DEFICIENCIES.

16.2.4. PERFORM VISUAL INSPECTION OF ALL WELDS, INCLUDING BUT NOT LIMITED TO FIT-UP, INTERMEDIATE PASSES, AND FINAL WELD.

16.2.5. PERFORM MAGNETIC PARTICLE INSPECTION ON 10% OF WELDS PER ASTM E 709. PERFORM ON ROOT PASS AND ON FINISHED WELD. CRACKS OR ZONES OF INCOMPLETE FUSION OR PENETRATION ARE NOT ACCEPTABLE. IF ANY PORTION OF A WELD DOES NOT CONFORM, 100% OF THE WELD FOR THAT MEMBER SHOULD BE TESTED.

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REVISION NO.	DESCRIPTION	DATE	BY:
REVISION NO. 0	DESCRIPTION ISSUE FOR BID	DATE 2/23/24	BY: JHB

ALABAMA LICENSED PROFESSIONAL ENGINEER
Jared Blount
No. 50027
PROFESSIONAL
02/23/2024
JARED H. BLOUNT

CITY OF FAIRHOPE
FAIRHOPE, ALABAMA

FLYING CREEK NATURE PRESERVE

GENERAL NOTES

thompson ENGINEERING
2970 COTTAGE HILL RD., STE. 190
MOBILE, ALABAMA 36606
TEL: (251) 666-2443
FAX: (251) 666-6422

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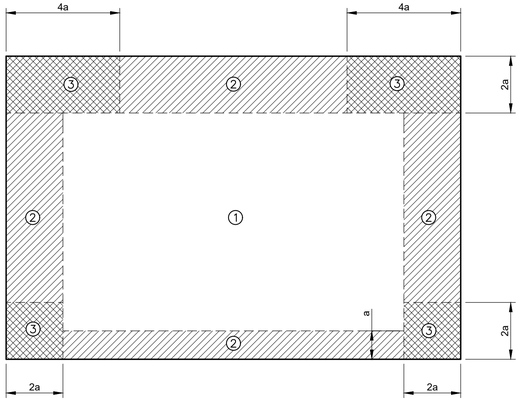
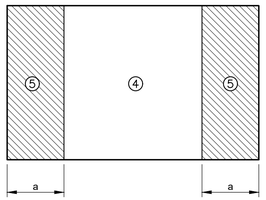


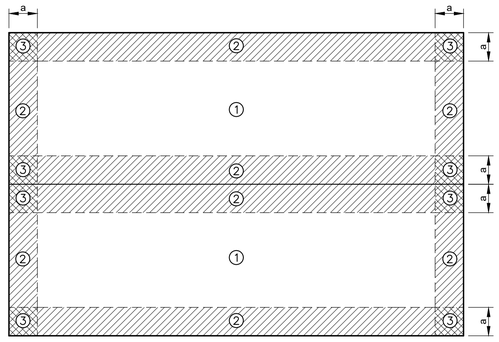
RESTROOM ROOF WIND DIAGRAM

ZONE 1
ZONE 2
ZONE 3



RESTROOM WALL WIND DIAGRAM

ZONE 4
ZONE 5



PAVILION/KIOSK ROOF WIND DIAGRAM

ZONE 1
ZONE 2
ZONE 3

RESTROOM COMPONENTS & CLADDING PRESSURES

	ZONE	AREA (SF)	ULTIMATE +P (PSF)	ULTIMATE -P (PSF)	ALLOWABLE +P (PSF)	ALLOWABLE -P (PSF)
ROOF	1	10	14.7	-57.5	8.8	-34.5
		20	13.1	-54.4	7.9	-32.6
		50	12.2	-48.3	7.3	-29.0
		100	11.6	-45.2	7.0	-27.1
		200	11.6	-42.2	7.0	-25.3
		500	11.6	-36.1	7.0	-21.7
	1'	10	14.7	-33	8.8	-19.8
		20	13.1	-33	7.9	-19.8
		50	12.2	-33	7.3	-19.8
		100	11.6	-33	7.0	-19.8
		200	11.6	-28.4	7.0	-17.0
		500	11.6	-22.3	7.0	-13.4
	2	10	14.7	-75.8	8.8	-45.5
		20	13.1	-71.2	7.9	-42.7
		50	12.2	-65.1	7.3	-39.1
		100	11.6	-60.5	7.0	-36.3
		200	11.6	-54.4	7.0	-32.6
		500	11.6	-48.3	7.0	-29.0
	3	10	14.7	-103.3	8.8	-62.0
		20	13.1	-94.2	7.9	-56.5
		50	12.2	-80.4	7.3	-48.2
		100	11.6	-71.2	7.0	-42.7
		200	11.6	-61.1	7.0	-36.7
		500	11.6	-48.3	7.0	-29.0
WALLS	4	10	36.1	-39.1	21.7	-23.5
		20	34.5	-37.6	20.7	-22.6
		50	31.5	-35.5	18.9	-21.3
		100	30.3	-33.6	18.2	-20.2
		200	29.3	-32.1	17.6	-19.3
		500	26.9	-30	16.1	-18.0
	5	10	36.1	-48.3	21.7	-29.0
		20	34.5	-45.2	20.7	-27.1
		50	31.5	-40.7	18.9	-24.4
		100	30.3	-37.6	18.2	-22.6
		200	29.3	-34.5	17.6	-20.7
		500	26.9	-30	16.1	-18.0

WIDTH OF EDGE STRIP, a = 4.20 FT

FOR ENCLOSURE CLASSIFICATION, SEE GENERAL NOTES

1. BASED ON ASCE 7-16, SECTION 30
2. ALLOWABLE WIND LOADS ARE 60% OF ULTIMATE WIND LOADS.

RESTROOM PAVILION COMPONENTS & CLADDING PRESSURES

	ZONE	AREA (SF)	ULTIMATE +P (PSF)	ULTIMATE -P (PSF)	ALLOWABLE +P (PSF)	ALLOWABLE -P (PSF)
ROOF	1	≤ a^2	13.1	-31.4	7.9	-18.9
		> a^2≤ 4.0a^2	13.1	-84.3	7.9	-50.6
		>4.0a^2	13.1	-84.3	7.9	-50.6
	2	≤ a^2	21.0	-47.2	12.6	-28.3
		> a^2≤ 4.0a^2	21.0	-131.7	12.6	-79.0
		>4.0a^2	13.1	-84.3	7.9	-50.6
	3	≤ a^2	26.2	-62.9	15.7	-37.7
		> a^2≤ 4.0a^2	21.0	-131.7	12.6	-79.0
		>4.0a^2	13.1	-84.3	7.9	-50.6

WIDTH OF EDGE STRIP, a = 3.00 FT

FOR ENCLOSURE CLASSIFICATION, SEE GENERAL NOTES

1. BASED ON ASCE 7-16, SECTION 30
2. ALLOWABLE WIND LOADS ARE 60% OF ULTIMATE WIND LOADS.

PAVILION COMPONENTS & CLADDING PRESSURES

	ZONE	AREA (SF)	ULTIMATE +P (PSF)	ULTIMATE -P (PSF)	ALLOWABLE +P (PSF)	ALLOWABLE -P (PSF)
ROOF	1	≤ a^2	34.0	-23.5	20.4	-14.1
		> a^2≤ 4.0a^2	20.9	-39.4	12.5	-23.6
		>4.0a^2	20.9	-39.4	12.5	-23.6
	2	≤ a^2	52.3	-36.6	31.4	-22.0
		> a^2≤ 4.0a^2	36.6	-55.1	22.0	-33.1
		>4.0a^2	20.9	-39.4	12.5	-23.6
	3	≤ a^2	67.9	-47.1	40.8	-28.2
		> a^2≤ 4.0a^2	36.6	-55.1	22.0	-33.1
		>4.0a^2	20.9	-39.4	12.5	-23.6

WIDTH OF EDGE STRIP, a = 3.88 FT

FOR ENCLOSURE CLASSIFICATION, SEE GENERAL NOTES

1. BASED ON ASCE 7-16, SECTION 30
2. ALLOWABLE WIND LOADS ARE 60% OF ULTIMATE WIND LOADS.

KIOSK COMPONENTS & CLADDING PRESSURES

	ZONE	AREA (SF)	ULTIMATE +P (PSF)	ULTIMATE -P (PSF)	ALLOWABLE +P (PSF)	ALLOWABLE -P (PSF)
ROOF	1	≤ a^2	13.1	-33.8	7.9	-20.3
		> a^2≤ 4.0a^2	13.1	-77.8	7.9	-46.7
		>4.0a^2	13.1	-77.8	7.9	-46.7
	2	≤ a^2	21.0	-50.8	12.6	-30.5
		> a^2≤ 4.0a^2	21.0	-121.4	12.6	-72.8
		>4.0a^2	13.1	-77.8	7.9	-46.7
	3	≤ a^2	26.2	-67.7	15.7	-40.6
		> a^2≤ 4.0a^2	21.0	-121.4	12.6	-72.8
		>4.0a^2	13.1	-77.8	7.9	-46.7

WIDTH OF EDGE STRIP, a = 3.00 FT

FOR ENCLOSURE CLASSIFICATION, SEE GENERAL NOTES

1. BASED ON ASCE 7-16, SECTION 30
2. ALLOWABLE WIND LOADS ARE 60% OF ULTIMATE WIND LOADS.

REVISION NO.	DESCRIPTION	DATE	BY:
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ALABAMA LICENSED PROFESSIONAL ENGINEER
No. 50027
02/23/2024
JARED H. BLOUNT

CITY OF FAIRHOPE
FAIRHOPE, ALABAMA

thompson ENGINEERING
2970 COTTAGE HILL RD, STE. 190
MOBILE, ALABAMA 36606
TEL: (251) 666-2443
FAX: (251) 666-6422

FLYING CREEK NATURE PRESERVE

COMPONENTS & CLADDING WIND PRESSURES

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SCHEDULE OF SPECIAL INSPECTIONS

SPECIAL CASES (IBC 1705.1.1)

ITEM	FREQUENCY	INSTRUCTIONS / COMMENTS

FABRICATORS (IBC 1704.2.5)

ITEM	FREQUENCY	EXTENT / COMMENTS
REVIEW THE FABRICATION AND QUALITY CONTROL PROCEDURES OF THE FOLLOWING FABRICATORS FOR COMPLETENESS AND ADEQUACY RELATIVE TO THE FABRICATOR'S SCOPE OF WORK: STEEL FABRICATOR, COLD-FORMED TRUSS FABRICATOR, WOOD TRUSS FABRICATOR, PRECAST CONCRETE SUPPLIER.	PERIODIC	FABRICATORS, IF REGISTERED AND APPROVED BY THE BUILDING OFFICIAL OR AN APPROVED AGENCY, SHALL SUBMIT REPORTS AND CERTIFICATES OF COMPLIANCE TO THE BUILDING OFFICIAL UPON COMPLETION OF FABRICATION. THE CERTIFICATE OF COMPLIANCE MUST STATE THAT FABRICATED ITEMS WERE CONSTRUCTED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

SOILS CONSTRUCTION (IBC 1705.6)

ITEM	FREQUENCY	EXTENT / COMMENTS
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	PERIODIC	AS RECOMMENDED IN APPROVED SOILS REPORT AND CONTAINED IN THE CONSTRUCTION DOCUMENTS.
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	PERIODIC	
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	PERIODIC	
DURING FILL PLACEMENT, VERIFY USE OF PROPER MATERIALS AND PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	CONTINUOUS	
PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	PERIODIC	

DRIVEN DEEP FOUNDATIONS (IBC 1705.7)

ITEM	FREQUENCY	EXTENT / COMMENTS
VERIFY ELEMENT MATERIALS, SIZES AND LENGTHS COMPLY WITH THE REQUIREMENTS.	CONTINUOUS	
DETERMINE CAPACITIES OF TEST ELEMENTS AND CONDUCT ADDITIONAL LOAD TESTS, AS REQUIRED.	CONTINUOUS	
OBSERVE DRIVING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH ELEMENT.	CONTINUOUS	
VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM TYPE AND SIZE OF HAMMER, RECORD NUMBER OF BLOWS PER FOOT OF PENETRATION, DETERMINE REQUIRED PENETRATIONS TO ACHIEVE DESIGN CAPACITY, RECORD TIP AND BUTT ELEVATIONS, AND DOCUMENT ANY DAMAGE TO FOUNDATION ELEMENTS.	CONTINUOUS	

CONCRETE CONSTRUCTION (IBC 1705.3)

ITEM	FREQUENCY	EXTENT / COMMENTS
ISOLATED SPREAD FOOTINGS ARE EXCEPTED FROM INSPECTIONS, BUT NOT MATERIALS TESTING.	---	
CONTINUOUS FOOTINGS ARE EXCEPTED FROM INSPECTIONS, BUT NOT MATERIALS TESTING.	---	
NON-STRUCTURAL SLABS ON GRADE (PATIOS, DRIVEWAYS, AND SIDEWALKS) AND PRESTRESSED SLABS ON GRADE WHERE THE EFFECTIVE PRESTRESS IN THE CONCRETE IS LESS THAN 150 PSI, ARE EXCEPTED FROM INSPECTIONS, BUT NOT MATERIALS TESTING.	---	
VERIFY TYPE, GRADE, SIZE, CLEANLINESS, LOCATION, PLACEMENT, AND SPACING OF REINFORCING STEEL. VERIFY LAP LENGTHS, BENDS, TIES, STIRRUPS AND CONNECTORS.	PERIODIC	
INSPECT ANCHORS TO BE CAST IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE.	PERIODIC	
VERIFY THAT CORRECT CONCRETE DESIGN MIX IS BEING USED.	PERIODIC	FOR EACH POUR.
PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STENGTH TESTS, PERFORM SLUMP TEST AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	CONTINUOUS	DURING PLACEMENT OPERATIONS. REFERENCE CONCRETE SPECIFICATIONS FOR SPECIFIC TESTS AND FREQUENCIES.
INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	CONTINUOUS	
VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC	MONITOR DURING HOT, COLD AND WINDY CONDITIONS. REFERENCE CONCRETE SPECIFICATIONS.
VERIFY IN-SITU CONCRETE STRENGTH PRIOR TO REMOVAL OF FORMS AND SHORES FROM BEAMS AND STRUCTURAL SLABS.	PERIODIC	PRIOR TO FORM OR SHORING REMOVAL.
INSPECT CONCRETE FORMWORK EXCEPT AS NOTED ABOVE FOR PROPER SHAPE, LOCATION AND DIMENSIONS. VERIFY THAT CONSTRUCTION JOINTS ARE PROPERLY KEYED. VERIFY THAT SLAB RECESSES, IF ANY, HAVE BEEN INSTALLED.	PERIODIC	PRIOR TO EACH POUR.
MEASURE FLOOR AND SLAB FLATNESS AND LEVELNESS ACCORDING TO ASTM E 1155.	PERIODIC	FOR EACH POUR. DO NOT SUBMIT REPORTS TO BUILDING OFFICIAL.

STRUCTURAL STEEL CONSTRUCTION (IBC 1705.2.1)

ITEM	FREQUENCY	EXTENT / COMMENTS
THE FABRICATORS QCI SHALL INSPECT THE FABRICATED STEEL TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE SHOP DRAWINGS. THIS INCLUDES SUCH ITEMS AS THE CORRRECT APPLICATION OF SHOP JOINT DETAILS AT EACH CONNECTION.	PERIODIC	THE ACCEPTANCE OR REJECTION OF JOINT DETAILS AND THE CORRECT APPLICATION OF JOINT DETAILS SHALL BE DOCUMENTED.
VERIFY STEEL FRAMES COMPLY WITH THE FIELD INSTALLED DETAILS SHOWN ON THE ERECTION DRAWINGS. VERIFY COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS FOR THE FOLLOWING: MEMBER LOCATIONS AND ORIENTATIONS, PROPER APPLICATION OF FIELD JOINT DETAILS AT EACH CONNECTION, AND BRACES AND STIFFENERS.	PERIODIC	THE ACCEPTANCE OR REJECTION OF JOINT DETAILS AND THE CORRECT APPLICATION OF JOINT DETAILS SHALL BE DOCUMENTED.
INSPECT ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS. THE DIAMETER, GRADE, TYPE AND LENGTH OF THE ANCHOR ROD OR EMBEDDED ITEM AND THE EXTENT OF DEPTH OF EMBEDMENT MUST BE VERIFIED AND DOCUMENTED PRIOR TO PLACEMENT OF CONCRETE.	PERIODIC	
PRIOR TO WELDING (AISC 360-16 TABLE N5.4-1)		
VERIFY AVAILABILITY OF WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS.	PERFORM	
VERIFY WELDING PROCEDURE SPECIFICATIONS (WPS) AND MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE.	PERFORM	
VERIFY TYPE AND GRADE OF MATERIAL.	OBSERVE	
MAINTAIN A SYSTEM BY WHICH THE WELDER OF A JOINT CAN BE IDENTIFIED.	OBSERVE	THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE.
FOR GROOVE WELDS: VERIFY JOINT PREPARATION, DIMENSIONS (ALIGNMENT ROOT OPENING, ROOT FACE, BEVEL), CLEANLINESS (CONDITION OF STEEL SURFACES), TACKING (TACK WELD QUALITY AND LOCATION), FIT-UP, JOINT GEOMETRY, AND BACKING TYPE AND FIT (IF APPLICABLE) OF GROOVE WELDS.	OBSERVE	
INSPECT FITUP OF CJP GROOVE WELDS OF HSS T-, Y- AND K-JOINTS WITHOUT BACKING (INCLUDING POINT GEOMETRY)		
VERIFY CONFIGURATION AND FINISH OF ACCESS HOLES.	OBSERVE	
VERIFY DIMENSIONS (ALIGNMENT, GAPS AT ROOT), CLEANLINESS (CONDITION OF STEEL SURFACES), AND TACKING (TACK WELD QUALITY AND LOCATION) OF FILLET WELDS.	OBSERVE	
CHECK WELDING EQUIPMENT	OBSERVE	
DURING WELDING (AISC 360-16 TABLE N5.4-2)		
PLACEMENT AND INSTALLATION OF STEEL HEADED ANCHOR STUDS	OBSERVE	
VERIFY PACKAGING AND EXPOSURE CONTROL OF WELDING CONSUMABLES.	OBSERVE	
VERIFY THAT WELDING DOES NOT OCCUR OVER CRACKED TACK WELDS.	OBSERVE	
VERIFY WIND SPEED, PRECIPITATION AND TEMPERATURE ARE WITHIN LIMITS.	OBSERVE	
VERIFY SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, WELDING MATERIALS, SHIELDING GAS TYPE / FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED AND PROPER POSITION.	OBSERVE	
VERIFY INTERPASS AND FINAL CLEANING, EACH PASS IS WITHIN PROFILE LIMITATIONS AND QUALITY REQUIREMENTS OF EACH PASS.	OBSERVE	
AFTER WELDING (AISC 360-16 TABLE N5.4-3)		
VERIFY WELDS ARE CLEANED.	OBSERVE	
VERIFY SIZE, LENGTH AND LOCATION OF WELDS.	PERFORM	
VERIFY WELDS MEET VISUAL ACCEPTANCE CRITERIA: CRACK PROHIBITION, WELD / BASE METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, SIZE, UNDERCUT, AND POROSITY.	PERFORM	
INSPECT WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILD UP HEAVY SHAPES. NO CRACK(S), AT ANY LOCATION, IS ACCEPTABLE.	PERFORM	AFTER HEAVY ROLLED SHAPES AND BUILT-UP HEAVY SHAPES ARE WELDED, VISUALLY INSPECT THE WELD ACCESS HOLE FOR CRACKS.
INSPECT ARC STRIKES, K-AREA, BACKING AND WELD TABS REMOVED, AND REPAIR ACTIVITIES.	PERFORM	WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3IN OR 75MM OF THE WELD.
DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT/MEMBER.	PERFORM	
NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR.	OBSERVE	
NON-DESTRUCTIVE TESTING (AISC 360 -16 SECTION N5.5)		
5/16" OR GREATER MATERIAL THICKNESS: 1) COMPLETE AND PARTIAL PENETRATION WELDS 2) MULTIPASS FILLET WELDS. 3) SINGLE-PASS FILLET WELDS > 1/4"	---	ULTRASONIC TESTING SHALL BE PERFORMED ON 10% OF JOINTS. TESTING RATE SHALL BE 100% IF GREATER THAN 5% OF TESTED WELDS HAVE UNACCEPTABLE DEFECTS. A TESTING OF AT LEAST 20 COMPLETED WELDS ON EACH PROJECT SHALL BE MADE PRIOR TO IMPLEMENTING SUCH AN INCREASE. IF THE REJECTION RATE FOR THE WELDER OR WELDING OPERATOR FALLS TO 5% OR LESS ON THE BASIS OF 40 COMPLETED WELDS, THE RATE OF ULTRASONIC TESTING MAY BE DECREASED TO 10%
DOCUMENT ALL NDT TESTING. WHEN A WELD IS REJECTED ON THE BASIS OF NDT, THE NDT RECORD SHALL INDICATE THE LOCATION OF THE DEFECT AND THE BASIS OF REJECTION.	---	REPORT SHALL IDENTIFY WELD BY LOCATION AND PIECE MARK FOR SHOP FABRICATION. FOR FIELD WORK, THE NDT REPORT SHALL IDENTIFY THE TESTED WELD BY LOCATION IN THE STRUCTURE, PIECE MARK, AND LOCATION IN THE PIECE.

WOOD CONSTRUCTION (IBC 1705.5)

ITEM	FREQUENCY	EXTENT / COMMENTS
INSPECT SITE-BUILT ASSEMBLIES INCLUDING SITE BUILT TRUSSES. INSPECT ERECTED TRUSSES INCLUDING BRIDGING AND ATTACHMENTS.	PERIODIC	

SPECIAL INSPECTION FOR WIND RESISTANCE (IBC 1705.12)

ITEM	FREQUENCY	EXTENT / COMMENTS
INSPECT FIELD GLUING OPERATIONS OF WOOD ELEMENTS OF THE MAIN WIND FORCE RESISTING SYSTEM.	CONTINUOUS	
OBSERVE NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF WOOD COMPONENTS WITHIN THE MAIN WIND FORCE RESISTING SYSTEM INCLUDING SHEAR WALLS, DIAPHRAGMNS, DRAG STRUTS, BRACES, SHEAR PANELS, AND HOLD-DOWNS.	PERIODIC	
OBSERVE ROOF COVERING, ROOF DECK AND ROOF FRAMING CONNECTIONS.	PERIODIC	

NOTE: THE INSPECTION AND TESTING AGENT(S) SHALL BE ENGAGED BY THE OWNER OR THE OWNER'S AGENT AND NOT BY THE CONTRACTOR OR SUBCONTRACTOR WHOSE WORK IS TO BE INSPECTED OR TESTED. ANY CONFLICT OF INTEREST MUST BE DISCLOSED TO THE BUILDING OFFICIAL PRIOR TO COMMENCING WORK. THE QUALIFICATIONS OF THE INSPECTION AGENT(S).

CONTINUOUS: THE INSPECTOR IS PRESENT WHEN AND WHERE THE WORK TO BE INSPECTED IS BEING PERFORMED.
PERIODIC: THE INSPECTOR IS INTERMITTENTLY PRESENT WHERE THE WORK TO BE INSPECTED HAS BEEN OR IS BEING PERFORMED.
OBSERVE: THE INSPECTOR IS TO OBSERVE THESE ITEMS ON A RANDOM BASIS.
PERFORM: THE INSPECTOR IS TO PERFORM THESE TASKS FOR EACH JOINT OR MEMBER.

CAST-IN-PLACE CONCRETE MIX SCHEDULE

APPLICATION	EXPOSURE CLASS	STRENGTH (PSI)	TYPE	W/C RATIO	SLUMP	AIR CONTENT	MAX AGGREGATE SIZE	MAX CONCRETE WEIGHT (PCF)
SPREAD FOOTINGS	F0, S0, P0, C0	5,000	NORMAL WT.	0.40	5" TO 7"	---	3/4"	---
UNLESS NOTED OTHERWISE	F0, S0, P0, C0	4,000	NORMAL WT.	0.48	4" TO 6"	---	3/4"	---

NOTES:
1. EXPOSURE CLASS FOR FREEZE/THAW, SULFATES, PERMEABILITY, AND CORROSION ARE PER ACI 318, SECTION 4.2.
2. WHERE NO W/C RATIO, SLUMP, OR AIR CONTENT IS NOTED, CONCRETE MIX DESIGN SHALL BE AS RECOMMENDED BY THE READY MIX SUPPLIERS ENGINEER.
3. WHERE AIR ENTRAINMENT IS NOT REQUIRED PER THE ABOVE TABLE, THE CONTRACTOR, INSTALLER, OR SUPPLIER MAY CHOOSE TO INCLUDE AIR ENTRAINMENT TO IMPROVE PLACEMENT AND FINISHING CHARACTERISTICS. AIR ENTRAINMENT IS NOT PERMITTED IN NORMAL WEIGHT CONCRETE TO RECEIVE A HARD TROWEL FINISH , AND ENTRAPPED AIR SHALL NOT EXCEED 3%. AIR ENTRAINMENT IN LIGHT WEIGHT CONCRETE SLABS IS REQUIRED TO MEET FIRE RATING REQUIREMENTS. SLABS SHALL BE PROPERLY FINISHED TO AVOID SURFACE IMPERFECTIONS SUCH AS BLISTERING OR DELAMINATION.
4. CEMENT AND AGGREGATES SHALL BE FROM A SINGLE SOURCE.

CONCRETE TENSION SPLICE LAP LENGTHS

BAR SIZE	f'c=3000				f'c=4000				f'c=5000			
	TOP BARS		OTHER BARS		TOP BARS		OTHER BARS		TOP BARS		OTHER BARS	
	A	B	A	B	A	B	A	B	A	B	A	B
#3	22	28	17	22	19	25	15	19	17	22	13	17
#4	29	38	22	29	25	33	19	25	23	29	17	23
#5	36	47	28	36	31	41	24	31	28	36	22	28
#6	54	56	33	43	37	49	29	37	34	44	26	34
#7	63	81	48	63	54	71	42	54	49	63	38	49
#8	72	93	55	72	62	81	48	62	56	72	43	56
#9	81	105	62	81	70	91	54	70	63	81	48	63
#10	91	118	70	91	79	102	61	79	71	92	54	71
#11	101	131	78	101	87	114	67	87	78	102	60	78

NOTES:
1. ALL LENGTHS ARE IN INCHES.
2. BAR COVER AND TRANSVERSE REINFORCEMENT SHALL MEET CODE MINIMUM.
3. LAP SPLICING OF #14 & #18 BARS IS NOT ALLOWED.
4. LAP LENGTHS ARE FOR NORMAL WEIGHT CONCRETE WITH UNCOATED, 60 KSI BARS.
5. WHEN LAPPING BARS OF DIFFERENT SIZES USE THE SPLICE LAP LENGTH OF THE SMALLER BAR, OR THE DEVELOPMENT LENGTH OF THE LARGER BAR, WHICHEVER IS GREATER. THE "A" VALUE FROM THE TABLE IS EQUAL TO THE BAR DEVELOPMENT LENGTH.
6. TOP BARS ARE HORIZONTAL REINFORCEMENT WITH MORE THAN 12" OF CONCRETE CAST BELOW THE REINFORCEMENT.

CIP CONCRETE CLEAR COVER SCHEDULE

LOCATION			COVER (IN)
CONCRETE CAST AGAINST & EXPOSED TO EARTH			3"
CONCRETE EXPOSED TO EARTH OR WEATHER:			
	#6 TO #18 BARS		2"
	#5, w31, AND SMALLER BARS		1 1/2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:			
	SLABS, WALLS, AND JOISTS		
		#14 AND #18 BARS	1 1/2"
		#11 AND SMALLER BARS	3/4"
	BEAMS AND COLUMNS		1 1/2"
FOOTINGS, GRADE BEAMS, AND PILE CAPS			2" TOP
			3" BOT. & SIDES
PEDESTALS AND COLUMNS			1 1/2" CLEAR OF TIES
ELEVATED SLABS EXPOSED TO WEATHER:			
	#5 AND SMALLER BARS		1 1/2" TOP & 3/4" BOT.
	#6 AND GREATER BARS		2" TOP & 3/4" BOT.
WELDED WIRE REINFORCEMENT:			
	5" OR LESS SLAB THICKNESS		CENTER
	6" OR GREATER SLAB THICKNESS		2" FROM TOP
BEAMS			1 1/2" CLR OF STIRRUPS
JOISTS			1 1/2" ALL SIDES

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ALABAMA LICENSED — No. 50027 — PROFESSIONAL — 02/23/2024 — ENGINEER — JARED H. BLOUNT

CITY OF FAIRHOPE
FAIRHOPE, ALABAMA

thompson ENGINEERING
2970 COTTAGE HILL RD., STE. 190
MOBILE, ALABAMA 36606
TEL: (251) 666-2443
FAX: (251) 666-6422

FLYING CREEK NATURE PRESERVE

SCHEDULE AND TABLES

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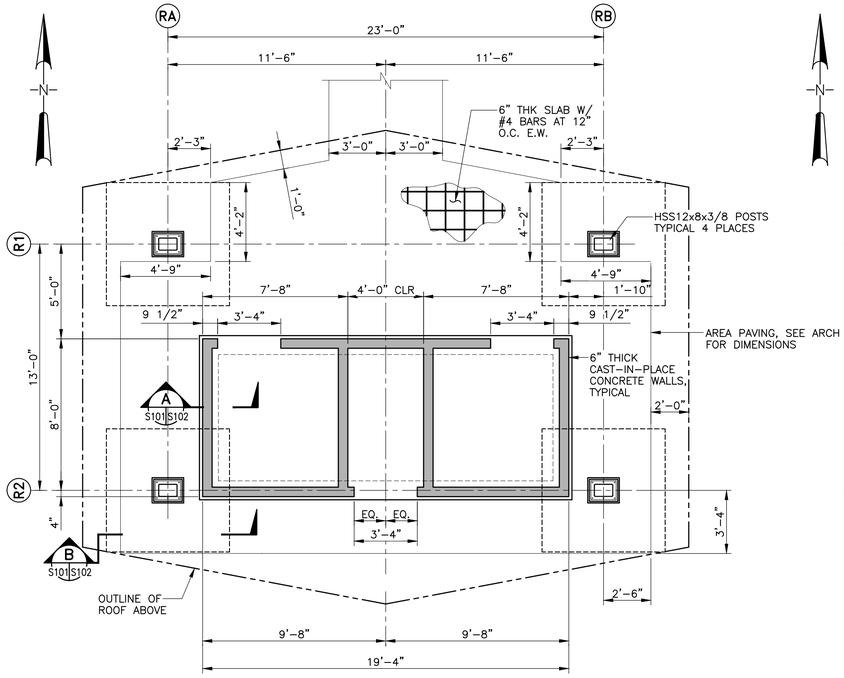
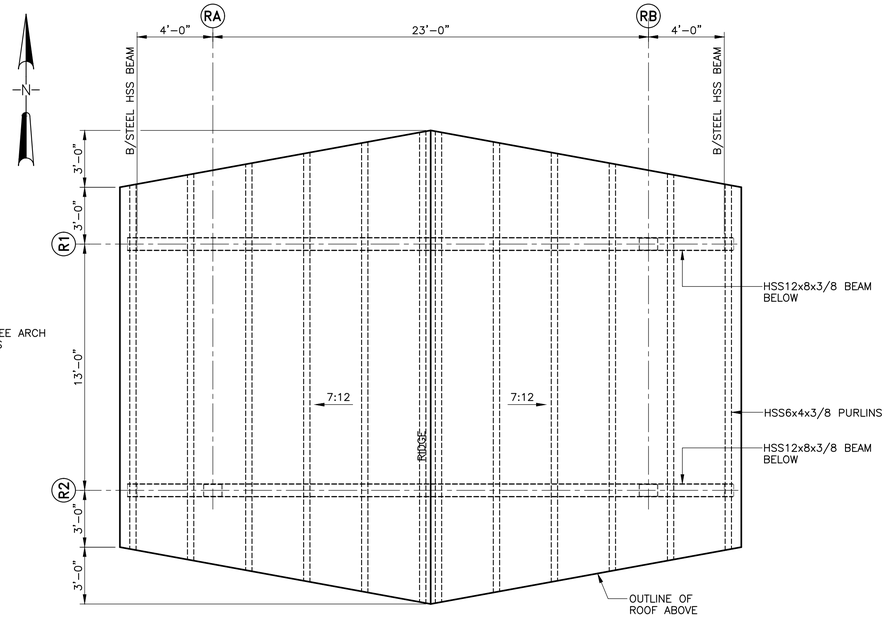


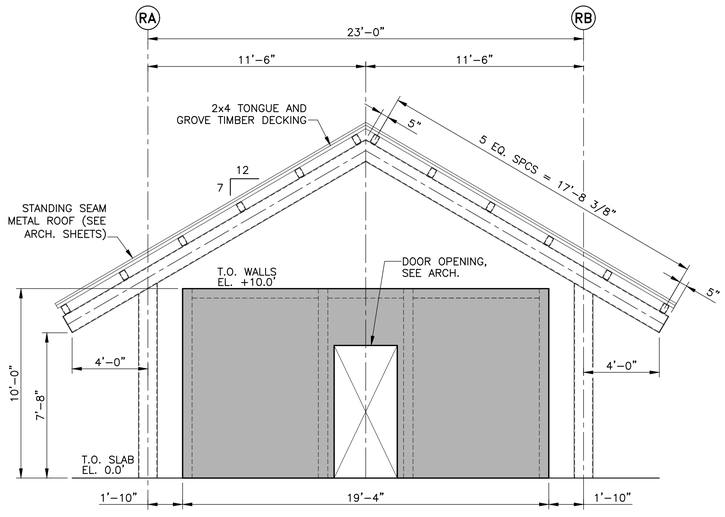
RESTROOM FOUNDATION PLAN

SCALE: 1/4" = 1'-0"



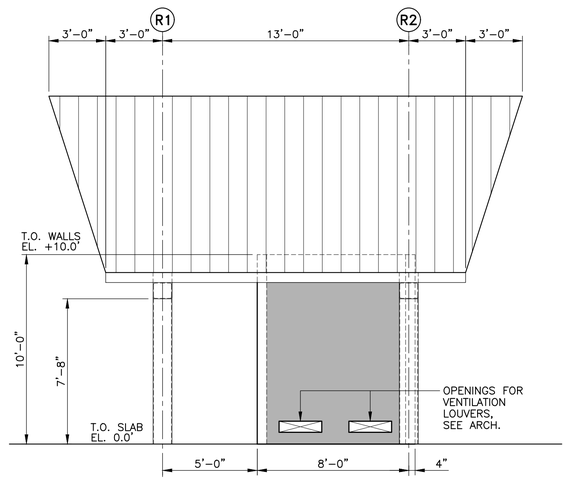
RESTROOM ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"



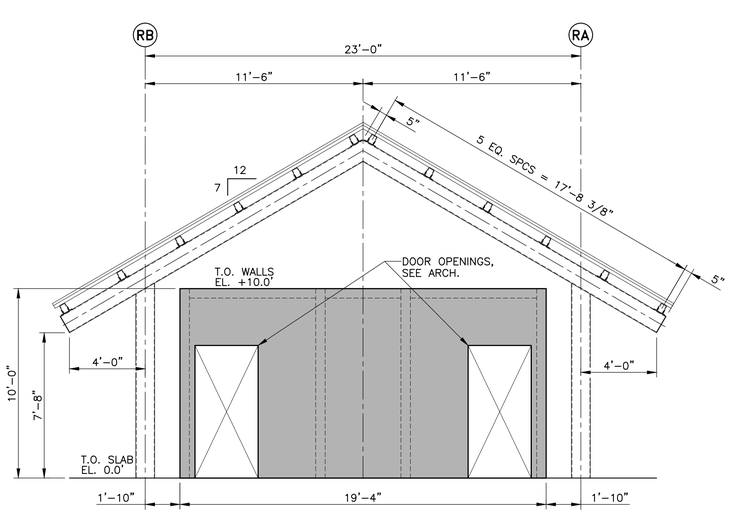
SOUTH ELEVATION

SCALE: 1/4" = 1'-0"



WEST ELEVATION (SHOWN)
EAST ELEVATION (OPPOSITE HAND)

SCALE: 1/4" = 1'-0"



NORTH ELEVATION

SCALE: 1/4" = 1'-0"

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CITY OF FAIRHOPE
FAIRHOPE, ALABAMA

FLYING CREEK NATURE PRESERVE

thompson ENGINEERING
2970 COTTAGE HILL RD, STE. 190
MOBILE, ALABAMA 36606
TEL: (251) 666-2443
FAX: (251) 666-6422

RESTROOM
PLANS AND ELEVATIONS

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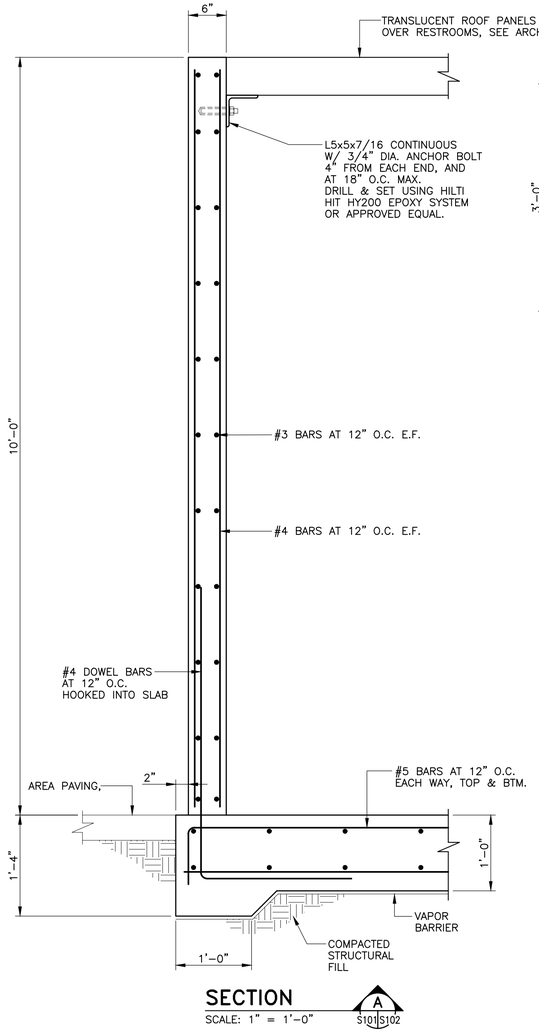
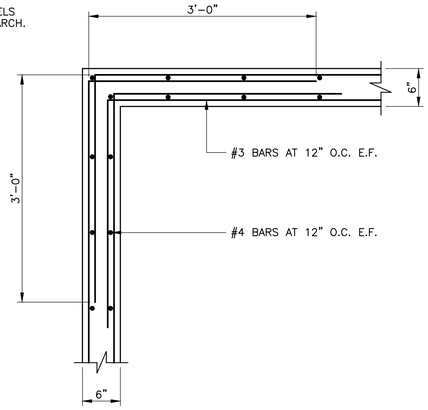


SECTION

SCALE: 1" = 1'-0" — A — S101|S102



REINFORCEMENT DETAIL AT WALL CORNERS

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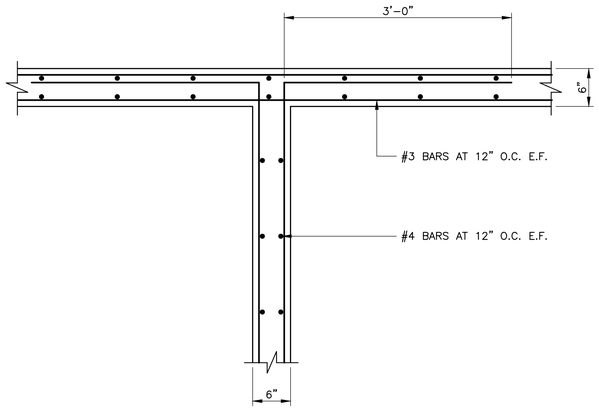

REINFORCEMENT DETAIL AT WALL INTERSECTIONS

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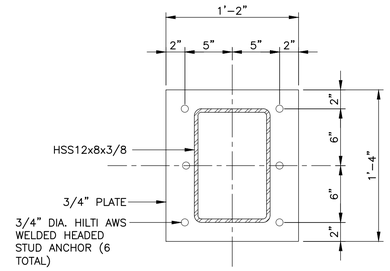
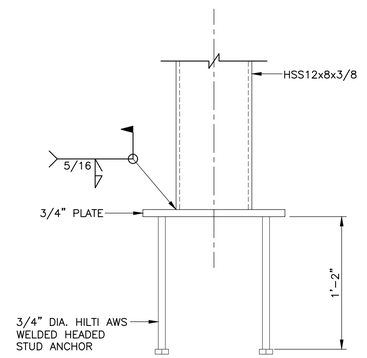


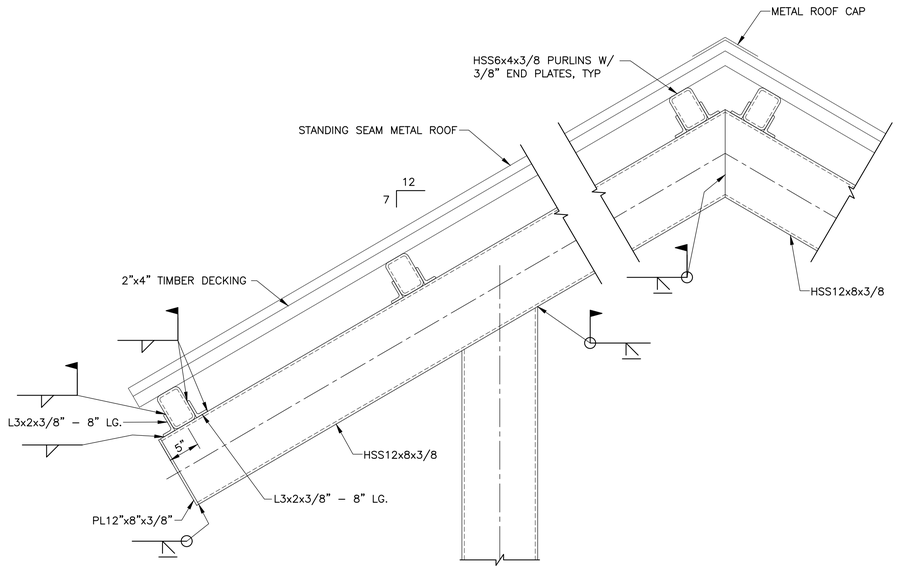
PLAN



ELEVATION

BASE PLATE DETAIL

SCALE: 1 1/2" = 1'-0"



FRAME DETAIL

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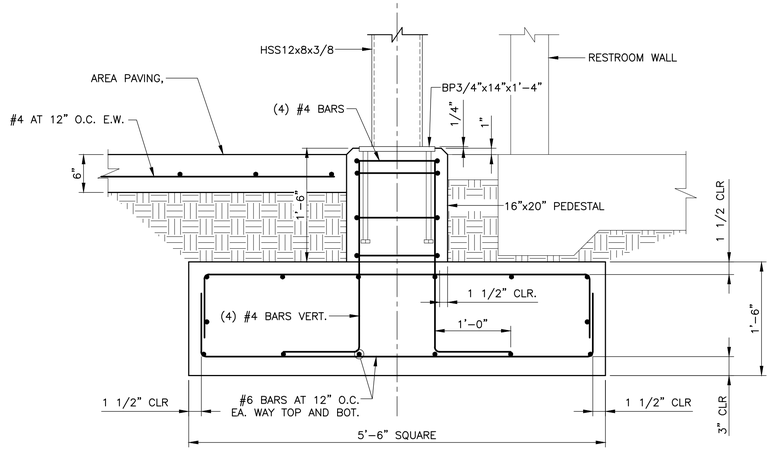

SECTION

SCALE: 1" = 1'-0" — B — S101|S102

NOTE: FOOTING AND PEDESTAL TO BE 5,000 PSI CONCRETE.

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CITY OF FAIRHOPE
FAIRHOPE, ALABAMA

FLYING CREEK NATURE PRESERVE

RESTROOM
SECTIONS AND DETAILS

thompson ENGINEERING
2970 COTTAGE HILL RD., STE. 190
MOBILE, ALABAMA 36606
TEL: (251) 666-2443
FAX: (251) 666-6422

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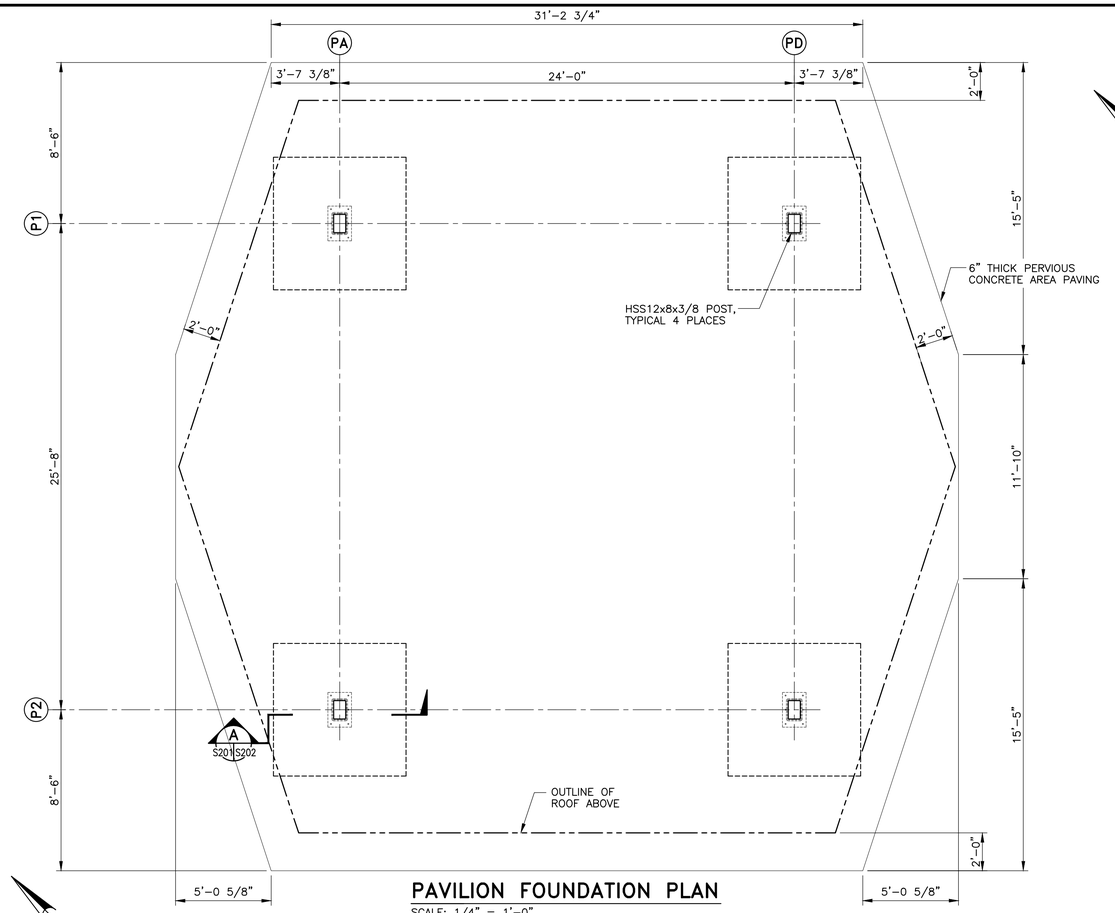
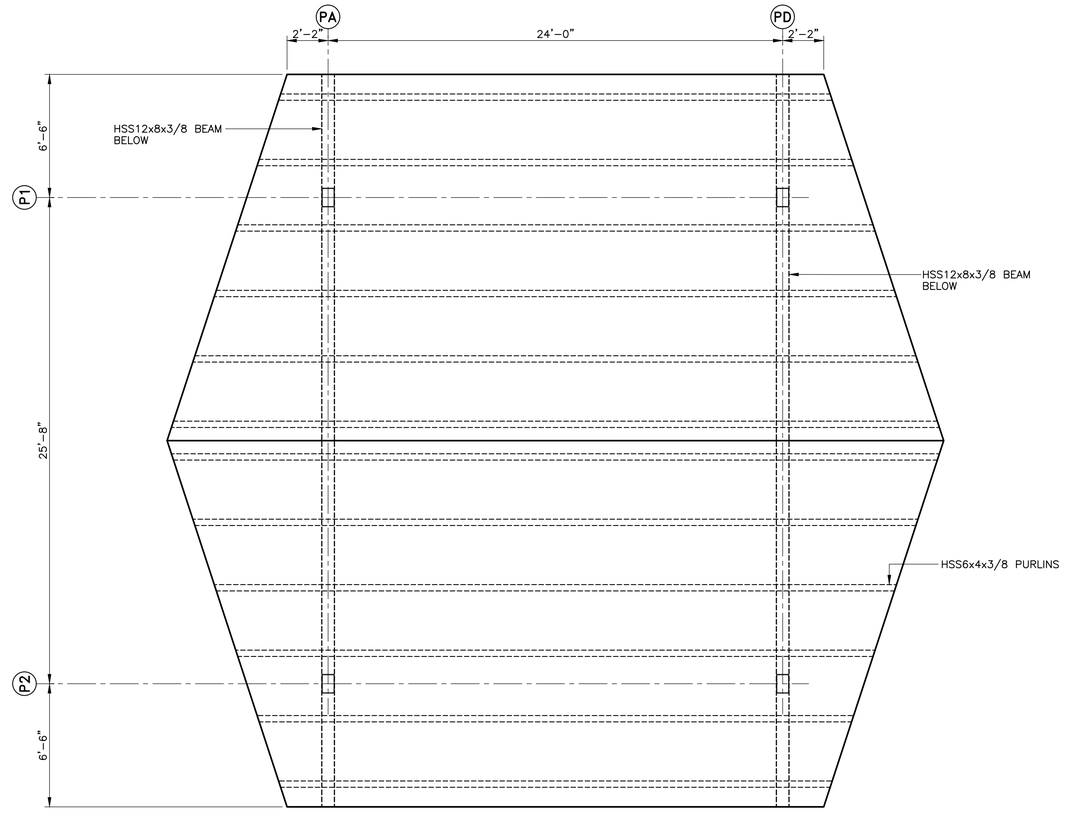


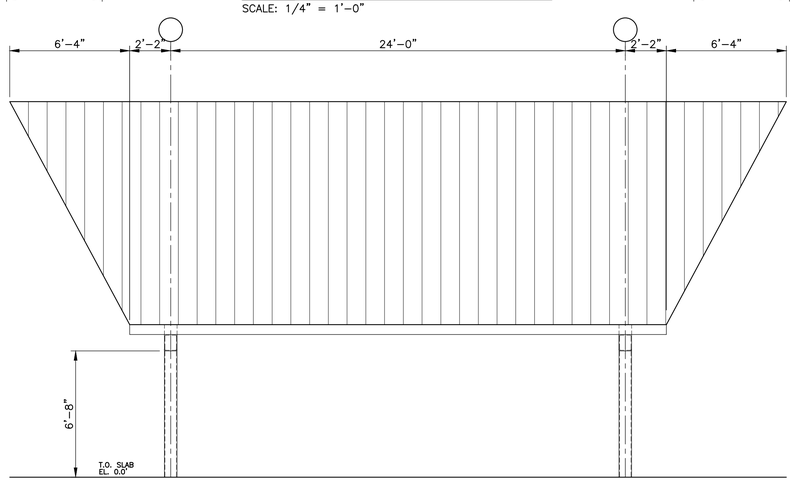
PAVILION FOUNDATION PLAN

SCALE: 1/4" = 1'-0"



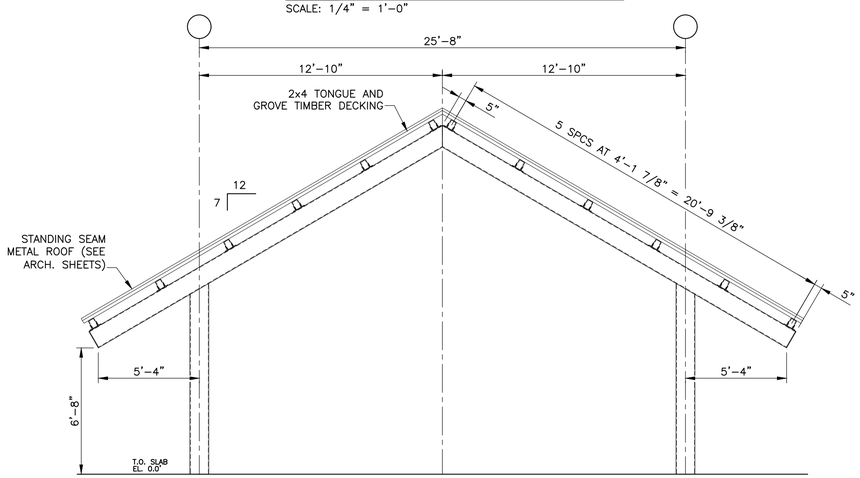
PAVILION ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"



NORTH AND SOUTH ELEVATION

SCALE: 1/4" = 1'-0"



EAST AND WEST ELEVATION

SCALE: 1/4" = 1'-0"

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FAIRHOPE, ALABAMA

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thompson ENGINEERING
2970 COTTAGE HILL RD., STE. 190
MOBILE, ALABAMA 36606
TEL: (251) 666-2443
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PAVILION
PLANS AND ELEVATIONS

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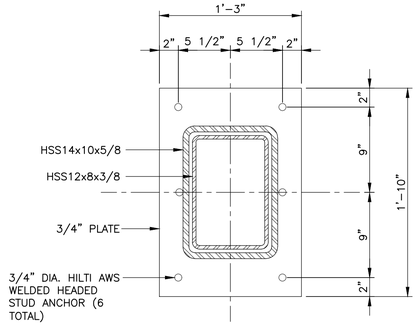
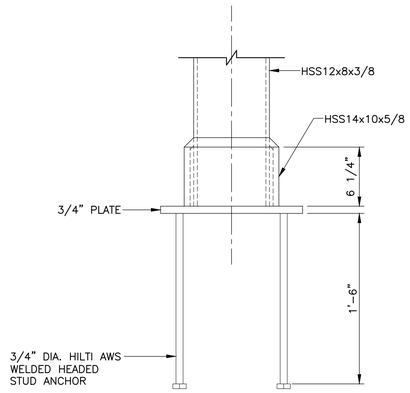


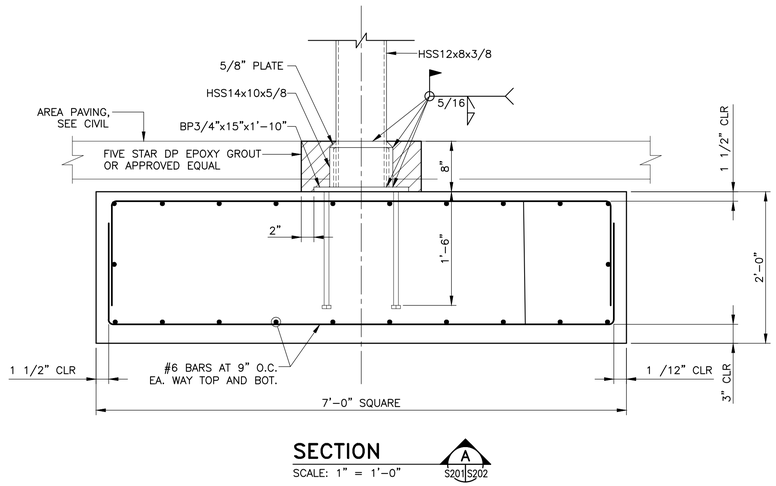
PLAN



ELEVATION

BASE PLATE DETAIL

SCALE: 1 1/2" = 1'-0"

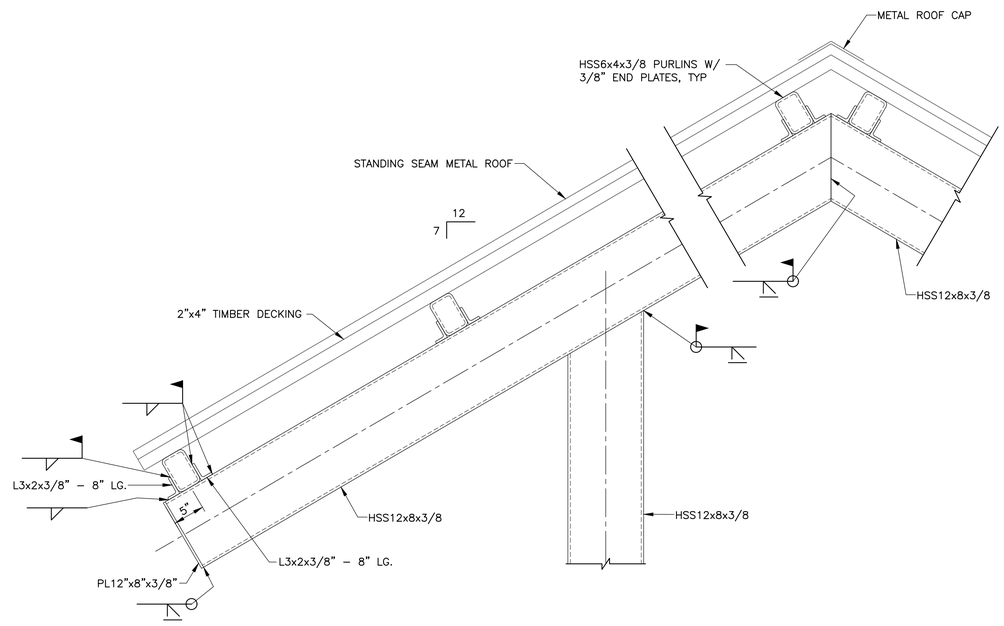


SECTION

SCALE: 1" = 1'-0"

A S201 S202

NOTE: FOOTING AND PEDESTAL TO BE 5,000 PSI CONCRETE.



FRAME DETAIL

SCALE: 1" = 1'-0"

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REVISION NO.	DESCRIPTION	DATE	BY:
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ALABAMA LICENSED No. 50027 PROFESSIONAL ENGINEER JARED H. BLOUNT 02/23/2024

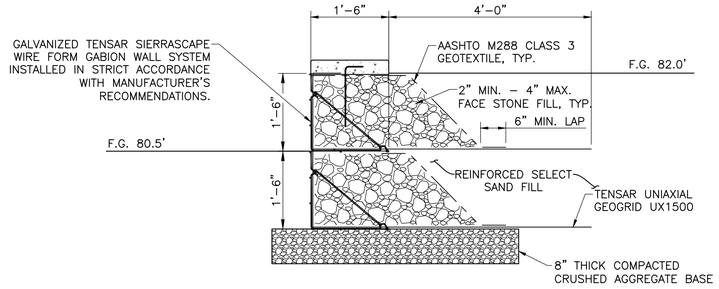
CITY OF FAIRHOPE
FAIRHOPE, ALABAMA

thompson ENGINEERING
2970 COTTAGE HILL RD., STE. 190
MOBILE, ALABAMA 36606
TEL: (251) 666-2443
FAX: (251) 666-6422

FLYING CREEK NATURE PRESERVE

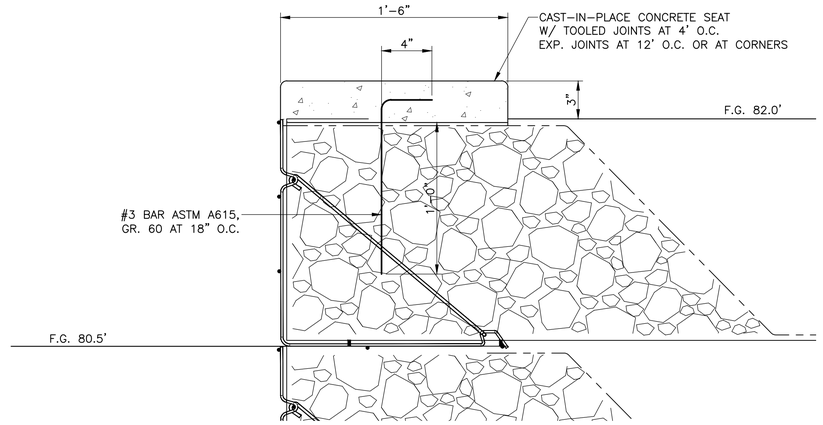
PAVILION
SECTIONS AND DETAILS

SCALE:	PLOT SCALE:	DRAWN BY:	CHECKED BY:	APPROVED BY:	DATE :	JOB NO. :	DRAWING NO.	REVISION NO. :
NOTED	1:1	RWA	RAH	JHB	FEBRUARY 2024	22-1101-0229	S202	0



SEAT WALL TYPICAL SECTION

SCALE: N.T.S.



SEAT CAP DETAIL

SCALE: N.T.S.

SEAT CAP NOTES

1. DESCRIPTION: CUSTOM RECYCLED GLASS CONCRETE SEAT CAP INSTALLED ATOP TENSAR SIERRASCAPE GABION WALL AS SHOWN ON THE PLANS. SEAT CAPS TO BE MADE FROM 70% RECYCLED CRUSHED GLASS CONCRETE SOURCED FROM THE CITY OF FAIRHOPE.
2. MANUFACTURER: OSPREY INITIATIVE, LLC, 2350 HALLS MILL ROAD, MOBILE, ALABAMA, 36606, CONTACT: DON BATES, (O.) 251-525-9727, (C) 601-842-7305, WEBSITE: WWW.OSPREY.WORLD, EMAIL: DON.BATES@OSPREY.WORLD.
3. MODEL: CUSTOM ACCORDING TO PLANS.
4. QUANTITY: AS SHOWN ON PLANS.
5. FINISHES: CUSTOM ACCORDING TO PLANS.
6. INSTALLATION: PER LAYOUT AND SECTION SHOWN ON PLANS.

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REVISION NO.	DESCRIPTION	DATE	BY:
REVISION NO.	DESCRIPTION	DATE	BY:
REVISION NO.	DESCRIPTION	DATE	BY:
REVISION NO. 0	DESCRIPTION ISSUE FOR BID	DATE 2/23/24	BY: JHB

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SEAT WALL TYPICAL SECTIONS

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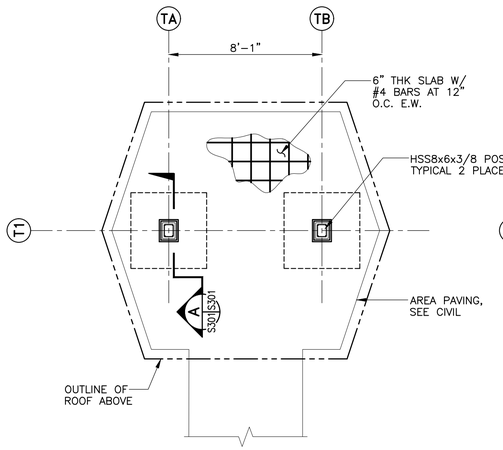
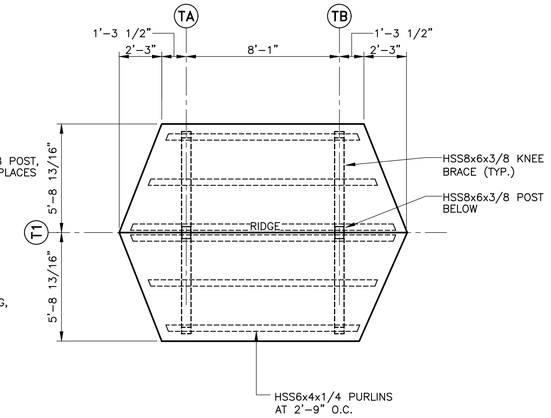
NOTED	1:1	RWA	RAH	JHB	FEBRUARY 2024	22-1101-0229	S203	0



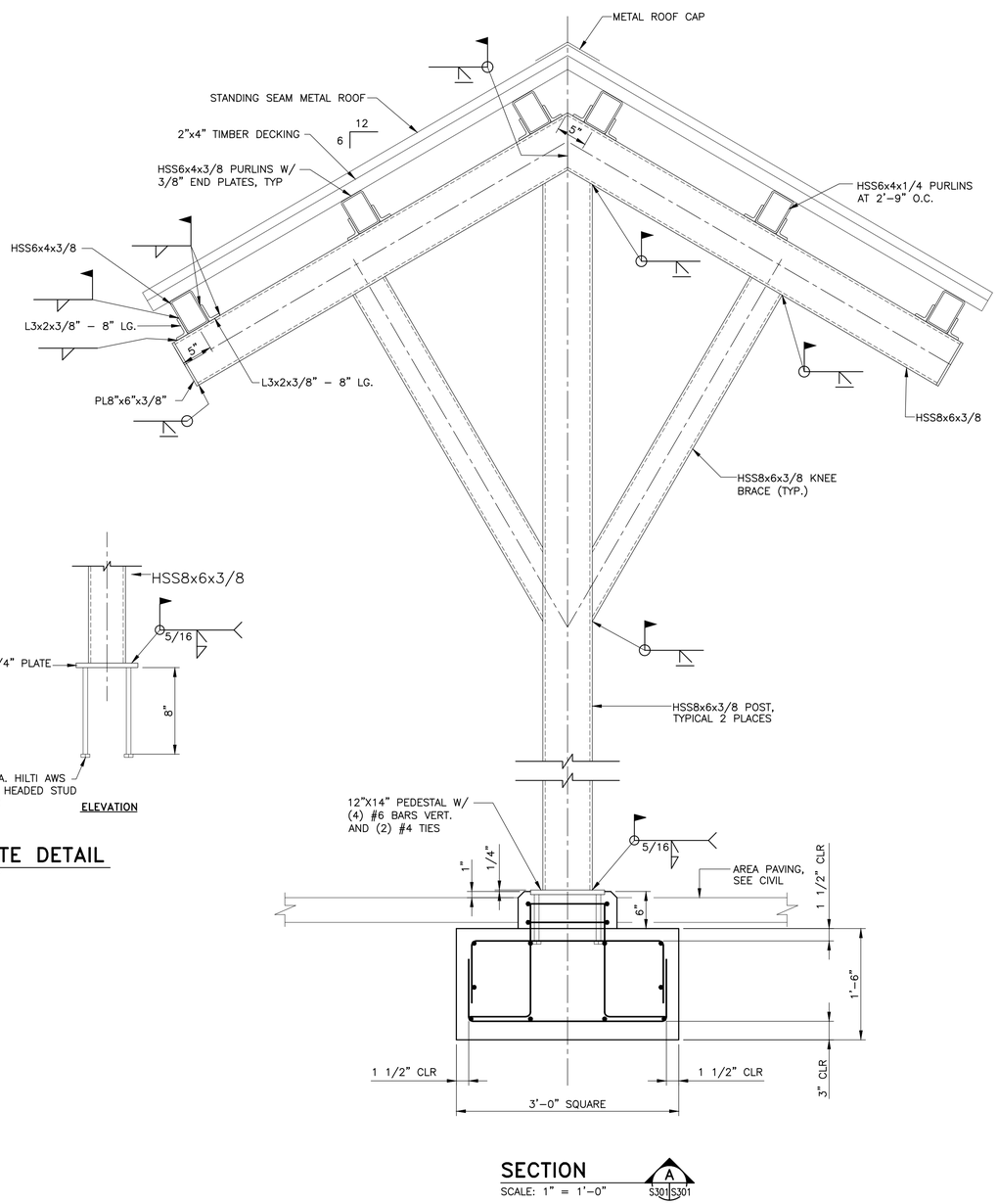
KIOSK FOUNDATION PLAN

SCALE: 1/4" = 1'-0"



KIOSK ROOF FRAMING PLAN

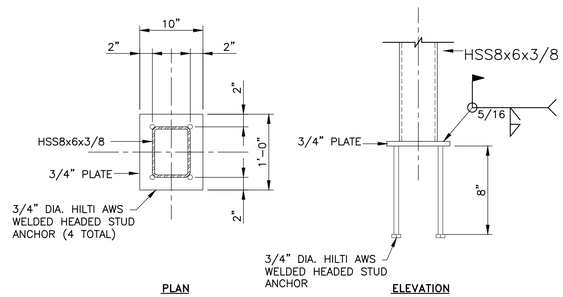
SCALE: 1/4" = 1'-0"



SECTION

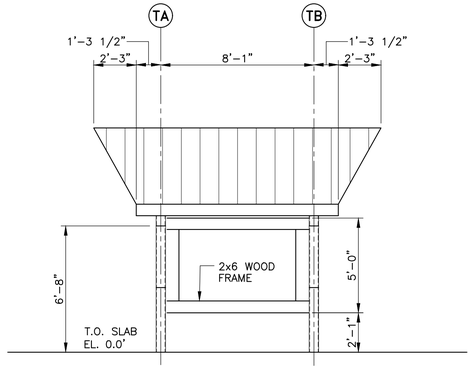
SCALE: 1" = 1'-0"

A S301 S301



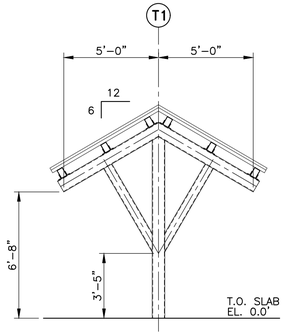
BASE PLATE DETAIL

SCALE: 1" = 1'-0"



FRONT ELEVATION

SCALE: 1/4" = 1'-0"



SIDE ELEVATION

SCALE: 1/4" = 1'-0"

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FAIRHOPE, ALABAMA

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KIOSK
PLANS AND ELEVATIONS

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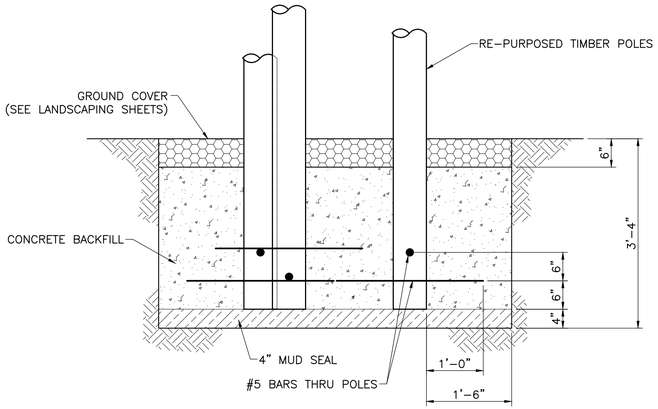
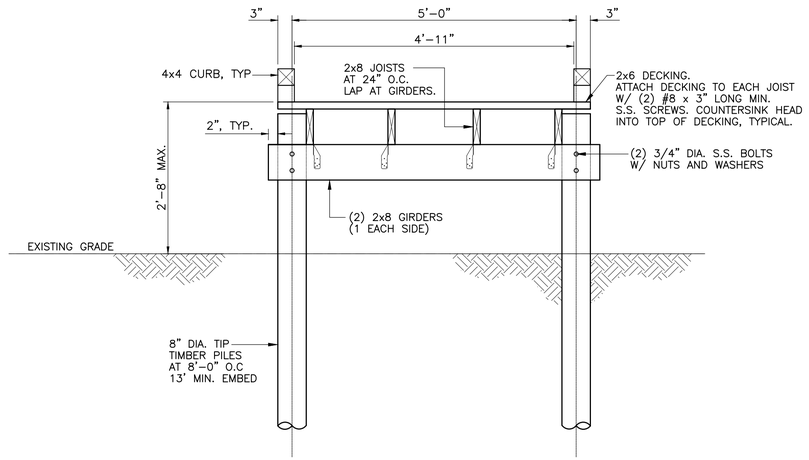
NOTED	1:1	RWA	RAH	JHB	FEBRUARY 2024	22-1101-0229	S301	0



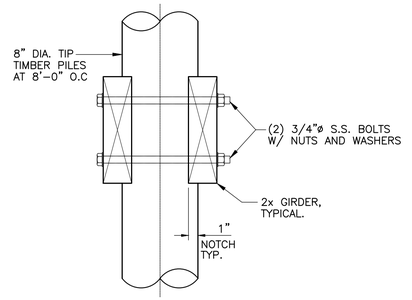
TYPICAL BIRD BLIND DETAIL

SCALE: 3/4" = 1'–0"



TYPICAL PEDESTRIAN BRIDGE SECTION

SCALE: 3/4" = 1'–0"



TYPICAL GIRDER CONNECTION

SCALE: 1 1/2" = 1'–0"

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REVISION NO.	DESCRIPTION	DATE	BY:
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PEDESTRIAN BRIDGE AND BIRD BLIND
TYPICAL SECTIONS AND DETAIL

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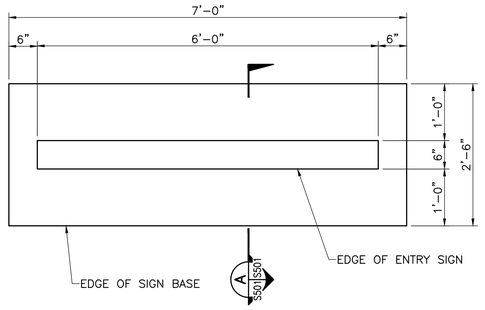
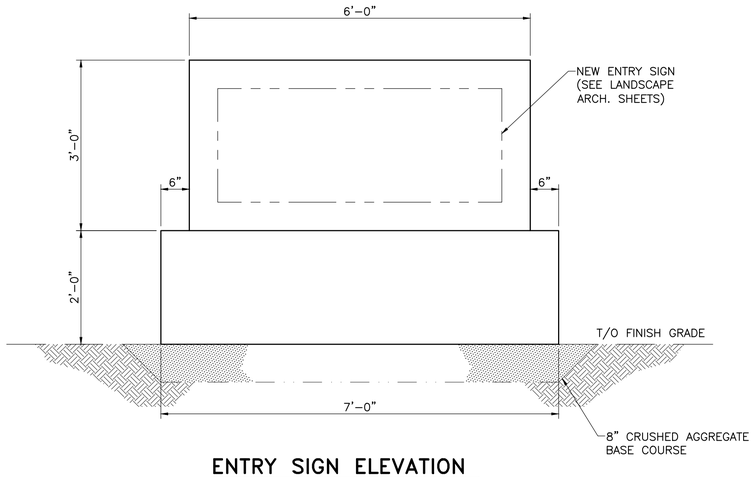
NOTED	1:1	RWA	RAH	JHB	FEBRUARY 2024	22–1101–0229	S401	0



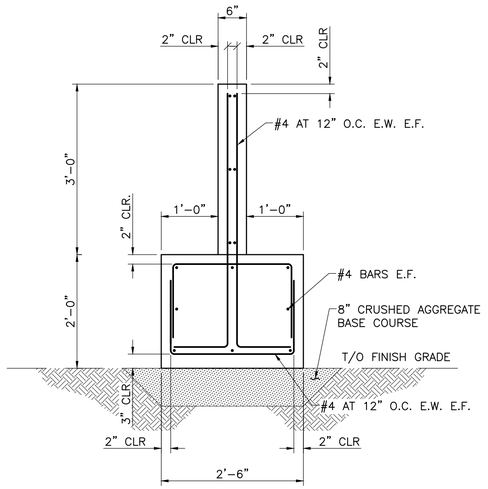
ENTRY SIGN PLAN

SCALE: 3/4" = 1'-0"



ENTRY SIGN ELEVATION

SCALE: 3/4" = 1'-0"



SECTION

SCALE: 3/4" = 1'-0"

A
S501 S501

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REVISION NO.	DESCRIPTION	DATE	BY:
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FLYING CREEK NATURE PRESERVE

ENTRY SIGN
PLAN, ELEVATION AND SECTION

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